

AD-A169 701

REPORT ON SEISMIC STABILITY ONONDAGA DAM NEW YORK
ATTACHMENT NUMBER 2 (U) CORPS OF ENGINEERS BUFFALO NY
BUFFALO DISTRICT T X GRASSO ET AL. MAY 86

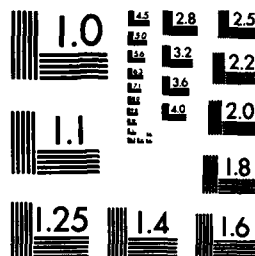
1/2

UNCLASSIFIED

DACW49-81-D-0011

F/G 13/2

ML



MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS-1963-A

AD-A169 701

(Date Entered)

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER	2. GOVT ACCESSION NO.	3. RECIPIENT'S CATALOG NUMBER
4. TITLE (and Subtitle) Report on Seismic Stability, Onondaga Dam, New York, Attachment No. 2.		5. TYPE OF REPORT & PERIOD COVERED
7. AUTHOR(s)		6. PERFORMING ORG. REPORT NUMBER
9. PERFORMING ORGANIZATION NAME AND ADDRESS U.S. Army Engineer District, Buffalo NCBED-DF 1776 Niagara St., Buffalo, NY 14207		8. CONTRACT OR GRANT NUMBER(s)
11. CONTROLLING OFFICE NAME AND ADDRESS		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS May 86
14. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office)		12. REPORT DATE May 1986
		13. NUMBER OF PAGES 357
		15. SECURITY CLASS. (of this report) Unclassified
		15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
16. DISTRIBUTION STATEMENT (of this Report) Approved for public release; distribution unlimited.		
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report) DTIC ELECTE JUL 11 1986 S B		
18. SUPPLEMENTARY NOTES		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number) Seismic Stability, Earth Dam, Flood Control Dam, Liquefaction, Dam Failure, Dam Stability, Seepage, Slope Protection, Periodic Inspection, Continuing Evaluation, Civil Works Projects. DTIC FILE COPY		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) This report is a reevaluation of the Stability of the existing Onondaga Dam which is located near the city of Syracuse, NY. This is an earth and rockfill dam and provides flood protection for the city of Syracuse which is downstream of the dam. This evaluation includes a seismic evaluation (Attachment 2) and a stability analysis based upon current Corps criteria. This document contains a Main Report, six appendices (A-F), and two attachments. The Main Report con- tains the local and regional geology at the project site and summarizes the results of the stability analysis. Appendix A contains pertinent data (Cont'd)		

DD FORM 1 JAN 73 1473 EDITION OF 1 NOV 65 IS OBSOLETE

86 7 7 051

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

SECURITY CLASSIFICATION OF THIS PAGE(When Data Entered)

20. (Cont'd)

about Onondaga Dam. Appendix B contains the rationale for the selected soil parameters used in the analysis. Appendix C contains hand calculations of the critical slip surfaces. Appendix D contains the computer input files and sample output for the slip circle analysis. Appendix E contains the wave runup analysis for the storage pool. Appendix F contains the slope protection calculations. Attachment No. 1 is the Concrete Spillway Stability analysis and Attachment No. 2 is the seismic evaluation.

SECURITY CLASSIFICATION OF THIS PAGE(When Data Entered)

ONONDAGA DAM, NY

REPORT ON SEISMIC STABILITY
ONONDAGA DAM, NY
ATTACHMENT NO. 2

U.S. Army Corps of Engineers, Buffalo District
1776 Niagara Street
Buffalo, NY

PREFACE

This study was conducted by H & A of New York and was authorized by the Department of the Army, Buffalo District, Corps of Engineers under Contract No. DACW 49-81-D-0011, Work Orders No. 2 and No. 3, dated October 13, 1981 and June 15, 1982, respectively.

The work was accomplished and the report written by Thomas X. Grasso, Staff Geologist, with assistance on seismologic evaluations provided by Dr. Edward F. Chiburis, Consultant, and Dr. Edward B. Kinner of Haley & Aldrich, Inc. All work was performed under the general technical supervision of Joseph J. Rixner, Manager of H & A of New York.

This work order was performed under the technical administration of the Buffalo District, Corps of Engineers, Geotechnical Section.



Accession For	
NTIS GRA&I	<input checked="checked" type="checkbox"/>
DTIC TAB	<input type="checkbox"/>
Unannounced	<input type="checkbox"/>
Justification	
By	
Distribution/	
Availability Codes	
Dist	Avail and/or Special
A-1	

FILE IN 1473 CORRECT
PER MR. THOMAS
VAN WART, ARMY
ENGINEERS BUFFALO NY

TABLE OF CONTENTS

	<u>Page</u>
PREFACE	i
TABLE OF CONTENTS	ii
LIST OF TABLES	iv
LIST OF FIGURES	v
PART I: INTRODUCTION	1
Purpose and Scope	1
General Description of Onondaga Dam	1
PART II: GEOLOGY	3
Physiography	3
Drainage	7
Central New York Stratigraphy	8
Onondaga Dam Site Stratigraphy	10
Depositional History	14
Glaciation	15
PART III: FIELD AND LABORATORY INVESTIGATIONS	17
Subsurface Exploration Program	17
Laboratory Testing Program	20
PART IV: SUBSURFACE CONDITIONS ALONG DAM CENTERLINE	21
Dam Embankment Materials	21
Fluvial Overbank Deposits	22
Deltaic Deposits	23
Lacustrine Deposits	24
Glacial Till	25
Bedrock	25
Subsurface Water	26
Subsurface Profile	26
PART V: TECTONIC HISTORY	28
Orogenies	28
Structural Deformations	30
Joints	32
Dikes	32

TABLE OF CONTENTS (continued)

	<u>Page</u>
PART VI: EARTHQUAKE ACTIVITY	34
Historic Earthquakes	34
Geographic Distribution of Earthquakes	34
PART VII: EVALUATION FOR ACTIVE FAULTS	36
Association of Earthquakes with Tectonism and Faults	36
Lineations	37
PART VIII: INTENSITY OF EARTHQUAKE SHAKING AT ONONDAGA DAM	38
Earthquake Intensities and Attenuation	38
Frequency of Earthquake Occurrence and Seismic Risk.	39
PART IX: ASSESSMENT OF EARTHQUAKE EFFECT ON ONONDAGA DAM	42
PART X: CONCLUSIONS	46
REFERENCES:	48
TABLES I - VI	
FIGURES 1 - 15	
APPENDIX A: CENTRAL NEW YORK SITE STRATIGRAPHY	
APPENDIX B: ONONDAGA DAM SITE STRATIGRAPHY	
APPENDIX C: ORIGINAL SUBSURFACE PROFILE	
APPENDIX D: TEST BORING LOGS D82-101 THROUGH D82-103	
APPENDIX E: LABORATORY TEST RESULTS	
APPENDIX F: ENGINEERING ANALYSES	
. SAMPLER HAMMER RATIOS	
. LIQUEFACTION ANALYSIS	

LIST OF TABLES

<u>Table</u>	<u>Title</u>
I	Geologic Time Scale
II	Generalized Stratigraphic Column - Central New York
III	Detailed Devonian Stratigraphic Column, Syracuse Meridian - Onondaga Dam Vicinity
IV	Earthquakes Within 200 km of Onondaga Dam Site
V	Chronological List of Significant ($I_0 \geq VI$) Earthquakes Within New York State
VI	List of Earthquakes Having Attenuated Intensities Greater Than or Equal to IV at the Onondaga Dam Site

LIST OF FIGURES

<u>Figure</u>	<u>Title</u>
1	Project Locus
2	Physiographic Provinces and General Bedrock Geology of New York State
3	Geologic Map of Central New York State
4	Site Geologic Map
5	Site and Subsurface Exploration Plan
6	Boring Location Plan and Subsurface Profile
7	Seismicity Epicenters, Northeastern U.S. Vicinity
7A	Seismic Zone Map of the Contiguous States and Puerto Rico
8	Intensity, Magnitude, and Acceleration Relationship
9	New York State Seismicity Zones
10	Isoseismals for I_0 = IX Event, LaMalbaie, Quebec
11	Attenuation Relationships
12	Probability of Exceedence for Peak Horizontal Ground Accelerations
13	Standard Penetration Resistance vs. Depth
14	Average Acceleration Spectra for Different Site Conditions
15	Liquefaction Potential for Dam Foundation Soils
15A	Liquefaction Potential of Dam Foundation Soils Correlated to Different Earthquake Magnitudes

GEOLOGICAL AND SEISMOLOGICAL INVESTIGATIONS

AT ONONDAGA DAM, NEW YORK

PART I: INTRODUCTION

Purpose and Scope

1. The purpose of this study was to investigate and evaluate the potential earthquake hazard at Onondaga Dam located in Central New York. This study was conducted in compliance with ER1110-2-1806 "Earthquake Design and Analysis for Corps of Engineers Dams" (by the Office of the Chief of Engineers, dated April 1977).

2. The study included geological, seismological, and subsurface investigations, and geotechnical engineering analyses. The study consisted of the following elements:

- a. an evaluation of the regional and local geology,
- b. performance of subsurface explorations and laboratory testing to further define the nature and density of soil deposits,
- c. an evaluation of the regional and local tectonic history with respect to structural deformation including faults,
- d. a review of historical regional seismicity,
- e. the determination of the maximum earthquake that will effect the site,
- f. an assessment of earthquake effects on Onondaga Dam, including an evaluation of liquefaction potential of the subsurface soils.

General Description of Onondaga Dam

3. Onondaga Dam is located on Onondaga Creek, a tributary to the Seneca River in Onondaga County, New York (Figure 1). It is about four miles southwest of the southern limits of the City of Syracuse, New York and about 3,000 feet south of N.Y. Route 80.

4. Onondaga Dam is a flood control structure consisting of a rolled earth fill embankment with a concrete overflow side channel spillway on the right (east) abutment. The overall length of the rolled fill embankment is 1,780 feet and the maximum height is 67 feet. An 1,100 ft. long spillway channel has been cut into rock along the right abutment. Rock cuts, ranging from about 15 to 40 ft. high, are exposed along the spillway channel.

5. The dam was completed in August 1949. It was designed and constructed by the Syracuse District of the U.S. Army Corps of Engineers.

PART II: GEOLOGY

Physiography

6. Figure 2 shows the physiographic provinces of New York State. Onondaga Dam is located in the northern part of the Allegheny (Appalachian) Plateau Physiographic Province. The physiographic province immediately north of the site is the Lake Plain Physiographic Province.

Lake Plain Province

7. Bordering Lakes Erie and Ontario is a region of low relief known as the Lake Plain Province. This plain is underlain by sedimentary rocks of Late Ordovician to Devonian age that dip gently southward at 30 to 50 feet per mile. Refer to Table I for a geologic time scale. This province is bordered on the South by the Appalachian Plateau with a more or less abrupt boundary.

8. Three escarpments or "cuestas" strike east-west across the Lake Ontario portion of the plain, especially west of Rochester to the Niagara Peninsula. They are, from north to south:

- a. Medina Escarpment - Formed in the resistant Grimsby and Thorold Sandstones. This ridge is of minor topographic relief being only some 30 to 50 feet high. Wherever streams cross this feature, waterfalls result such as those at Medina, Albion and Holley. The escarpment cannot be recognized east of the Genesee River. The line of the Barge Canal rests atop this escarpment from Spencerport to Lockport.
- b. Lockport or Niagara Escarpment - This ridge is the most prominent bedrock controlled topographic feature on the Lake Ontario Plain. The Lockport Dolostone forms the caprock of this "mountain ridge", the crest of which lies about 300 feet above Lake Ontario. Niagara Falls,

the Upper Falls of the Genesee at Rochester, and many other minor falls in between are also formed on the Lockport Dolostone.

- c. Onondaga Escarpment - The southernmost escarpment is about 50 feet in height, trending eastward from Buffalo through Batavia to Auburn. This escarpment is formed of resistant Onondaga Limestone. A line of abandoned and existing quarries marks its trace across the state, along with relatively small waterfalls on the numerous streams that cross the escarpment.

9. The remainder of the Lake Plain is underlain predominantly by shales, with a few relatively thin sandstone and carbonate interbeds forming the prominent east-west escarpments described above.

10. In general, the rocks are structurally simple, dipping gently southward and forming the north-northwest flank of the Appalachian (Allegheny) Synclinorium or Basin. This singly-dipping, homoclinal structure is interrupted locally by a few faults and a few minor low, open folds. The most significant of these is the Clarendon-Linden Fault, which strikes southwestward from Lake Ontario across northwest Monroe County and southeast Orleans County, through Batavia in eastern Genesee County and to Attica in central Wyoming County. It is a nearly vertical fault system, downthrown to the west, cutting the Precambrian basement, and having a displacement ranging from about 30 to 90 meters (100 to 275 ft.), (Hutchinson, et al. 1979).

11. A local fault structure of perhaps significant proportions occurs in northern Cayuga County but mainly in the subsurface, although one or more of these faults are thought to have a surface expression. Not much is known about this structure at present. A new gas field, the Blue Tail Rooster, is producing from traps in this structure. A well-developed, nearly vertical joint system of two sets, one essentially north-south and the other east-west is the only major, regional, geologic structure. Locally, a third set of joints may be present in places.

12. The Paleozoic rocks of the Lake Plain are mantled by a substantial thickness of glacial deposits. The most dramatic of these is the drumlin field of glacial till lying between Rochester and Syracuse. Several morainal deposits and glacial lake shoreline deposits, such as the offshore bar of Lake Iroquois are also present. Ridge Road (Rt. 104) runs along this bar.

Allegheney (Appalachian) Plateau

13. The Allegheny Plateau covers nearly a third of the entire State, as shown on Figure 2, and is its largest physiographic province. It is a region of moderate to high relief with summits at Elevations 2000 to 4000 feet above sea level. The Catskill Mountains make up the eastern portion of the plateau while the remainder is often referred to as the Southern Tier. The entire region is underlain by Devonian shales, siltstones, and sandstones. However, the tops of the hills, south of Olean, expose sandstones and conglomerates of Mississippian and Pennsylvanian age (Figure 2 and Table 1). The eastern border of this province is the Catskill Front, an escarpment composed of Middle Devonian, massive, red sandstone of the Catskill Delta. The entire Catskill region is held up by these coarse deltaic sandstones. The Helderberg Escarpment of Lower Devonian Limestones stretches from the Albany area westward to Syracuse forming the northern and eastern boundary. West of Syracuse, in the Finger Lakes and Genesee region, the boundary with the Lake Plain province is not abrupt, but more gradual, occurring in the northern portion of the Finger Lakes across the Middle Devonian Hamilton Group. In western New York and southeast of Lake Erie, the Portage Escarpment of Upper Devonian siltstones and sandstones forms the western border.

14. The strata of the Allegheny Plateau dip gently southward at about 50 to 80 feet per mile. In south central New York, gentle warping of the Devonian rocks occurred and produced several gentle open folds along with minor faulting. Deep wells and geophysical seismic surveys, penetrating through the Upper Silurian in this region, do not reveal

the large surface anticlines at depth. This suggests that the Upper Silurian salt beds may have "absorbed" the deformation. Aside from the Clarendon-Linden structure, no major faults are known in the Allegheny Plateau. A few small reverse faults of minor displacement are known in central New York, one in particular being the Portland Point Fault north of Ithaca along Cayuga Lake. Several thin, ultrabasic, peridotite dikes are known in central New York near Ithaca and Syracuse. These dikes have migrated upward along joint planes, are of Mesozoic age, and are of limited extent. The joint system is extremely regular, especially in Central New York, and comprises a north-south set and an east-west set. A third set is well developed only in the southwestern part of the province. Most of the streams in the bedrock gorges are controlled by the north-south and east-west joints.

15. Glacial deposits are relatively thin on the upland portions of this province, but the north-south valleys are normally deep U-shaped glacial troughs that are partially filled by thick moraine, lacustrine, and glacial outwash deposits. Some of the glacial troughs are plugged at both ends, thereby forming Finger Lakes, while other valleys have subsequently drained. A major east-west trending divide in drainage between the north flowing St. Lawrence and the south flowing Susquehanna systems occurs along the Valley Heads Moraine, which loops across the province at the southern extremities of the Finger Lakes and neighboring valleys.

16. In the Onondaga (Tully) Valley, the Tully Moraine, south of the site and west of the Village of Tully, is the local development of the Valley Heads Moraine. Onondaga Creek flows north off the moraine toward Lake Ontario and the Tioughnioga River flows south through Cortland to the Susquehanna River.

Drainage

17. The major drainage system through the site is Onondaga Creek, consisting of two branches; the west branch rising in the vicinity of Cedarville Village and the main branch rising at the Tully Moraine. Both branches join just south of the Onondaga Dam (Figures 3 and 4) and flow north past the dam into Onondaga Lake, a tributary of the Seneca River and part of the St. Lawrence System.

18. Onondaga Creek occupies a deep U-shaped glaciated valley that once impounded a finger lake in the waning stages of glaciation. Therefore the valley contains glaciofluvial sand and gravel as terraces on both sides probably representing hanging deltas. Lacustrine deposits floor the valley in the vicinity of the site and probably underlie the sands and gravels of the hanging deltas.

19. The highest delta terrace is approximately at El. 800 feet, just southeast of Nichols Corners, while the lowest is approximately at El. 520 feet near Nedrow.

Central New York Stratigraphy

20. The rock formations of central New York are structurally simple and abundantly fossiliferous, thereby lending themselves to detailed stratigraphic studies over the last 100 years. The stratigraphic sequence dips gently southward at approximately 40 to 50 feet per mile such that the oldest units are exposed in the north and progressively younger formations are exposed southward.

21. Table II presents the generalized stratigraphic column for central New York. Figure 3 is a geologic map of the central New York region.

22. A brief description of the various rock formations in central New York is contained in the following paragraphs. A more detailed discussion of thicknesses, lithologies, and relationships of the various series, groups and formations noted on Table II is presented in Appendix A.

Upper Ordovician System

23. The oldest stratigraphic unit in central New York, exposed along the south shore of Lake Ontario, is the Queenston Formation. It encompasses a maximum of 700 feet of red, crumbly shale and red siltstone and sandstone in this region. The coarser beds increase in thickness eastward and southward.

Silurian System

24. Silurian rocks surround the Appalachian Plateau on the north and are found in the Lake Ontario Plain, and on the east in the westernmost Hudson Valley Province. The older units of the Silurian sequence consist of the Grimsby Sandstone and Clinton and Lockport Groups of the Albion and Niagaran Series. These units are overlain by shales, dolomites and limestones of the Cayuga Series.

Devonian System

25. Devonian rocks are by far the most extensively exposed in New York State. Devonian strata underlie all of the Allegheny Plateau and regions adjacent to it, such as the Lake Erie Plain, southern Lake Ontario Plain, and the westernmost Hudson Valley Province.

26. Carbonates dominate the Lower Devonian and lower part of the Middle Devonian while clastic deposits predominate in the remainder of the section. The clastics are fine-textured marine rocks at the bottom, grading upward to coarse, red continental sandstones. These deposits, are known as the Catskill Delta and are a wedge of clastic sediments that thicken and become coarser to the east. The New York Devonian is subdivided into the Ulsterian Series (Lower Devonian), Erian Series (Middle Devonian) and Senecan and Chautauquan Series (Upper Devonian).

Onondaga Dam Site Stratigraphy

27. The stratigraphic sequence in the vicinity of the dam belongs to the Lower and Middle Devonian System described on Table II. A more detailed stratigraphic column along the Syracuse Meridian in the vicinity of the dam is summarized on Table III, and Figure 4 is a geologic map of the area surrounding the dam.

28. Older Silurian units are exposed to the north of the dam in Syracuse and younger Upper Devonian units cap the hills south of the dam, and form the valley sides and floor south of the Village of Tully.

29. A brief description of the stratigraphy exposed at the location of the dam is contained in the following paragraphs. Appendix B presents an expanded discussion of this stratigraphy, along with a discussion of the underlying or surrounding stratigraphy in the general vicinity of the dam site, as shown on Figure 4.

Lower Devonian - Ulsterian Series

Helderberg Group

30. The Helderberg group is composed of two formations; the Manlius Formation overlying the Rondout Formation (Chrysler Dolostone).

31. The Manlius Formation is subdivided, from older to younger, into the Olney Limestone Member, the Elmwood A, B, and C Dolostone Members, the Clark Reservation Limestone Member, and the Jamesville Limestone Member.

- a. The Olney Member is approximately 30 ft. of fine-grained, even bedded limestones; thin bedded in the lower portion, somewhat more thickly bedded in the upper portion.
- b. The Elmwood Member overlies the Olney and consists of a series of dolostones, waterlimes, and limestones. It is subdivided into three units. Elmwood A is a drab yellowish-brown, thin-bedded and mud cracked waterlime approximately 6 ft. thick. Elmwood B is a fine-grained,

blue limestone, approximately 3 ft. thick. Elmwood C, the uppermost unit, is approximately 4 ft. thick, and is a waterlime similar to Elmwood A.

- c. The Clark Reservation Limestone Member overlies the Elmwood and consists of a fine-grained, white weathering limestone characterized by a diagonal fracture system. It is approximately 5 ft. thick.
- d. The Jamesville Limestone Member overlies the Clark Reservation Member. It is approximately 25 ft. of fine-grained, dark blue, brittle limestone in thin even beds. It is exposed in the lower 12 ft. of the rock exposed in the spillway cut at the east end of the dam.

Tristates Group

32. The Oriskany Sandstone is the only formation of this group exposed in Central New York. It is not well developed in the vicinity of the dam. Only a few inches of reddish-orange, sandy layers of this unit are exposed at the base of the Onondaga Limestone in the spillway cut.

Middle Devonian - Erian Series

Onondaga Limestone

33. The Onondaga Limestone is the lowest Middle Devonian formation within the Syracuse area. It is a series of light bluish-gray semi-crystalline limestones in even continuous layers from 1 inch to 2 feet thick, separated by thin seams of dark calcareous shale. Flattened nodules of dark blue or black chert, sometimes in continuous sheets or beds, are unevenly distributed throughout the formation, but occur mainly in the upper part of the formation. The total formation thickness is approximately 70 ft.

34. The Onondaga is comprised of five members which, from older to younger, are the Edgecliff Member, the Nedrow Member, the Moorehouse Member, the Tioga Bentonite Member, and the Seneca Member.

- a. The Edgecliff Member is a coarse, crystalline limestone with abundant fossils. The base of the member is a disconformity; i.e., it was deposited over an erosion surface that had removed previously deposited rock strata. The Member is approximately 15 ft. thick and may be seen in the spillway cut.
- b. The Nedrow Member is characterized by thin-bedded, shaley limestone. It is medium gray, very fine-grained, and contains a little chert unevenly distributed at top. It is approximately 10 ft. thick and is gradational with the overlying Moorehouse Member. Both the Nedrow and the Moorehouse units are exposed in the spillway cut.
- c. The Moorehouse Member is a medium gray, very fine-grained limestone, with 2-in. to 5-ft. thick beds separated in many places by thin shaley partings. Total thickness of the deposit is approximately 25 ft. Chert is common in the Member, but varies in amount.
- d. The Tioga Bentonite Member is a volcanic ash layer 6 to 9 inches thick separating the Moorehouse and Seneca Members. Neither the Tioga nor the overlying Seneca Member were observed in the spillway cut.
- e. The Seneca Member is similar in lithology to the upper portion of the Moorehouse Member, but is characterized by a slightly different fossil assemblage. Chert is less common in the upper part of the Member and the limestone beds become darker gray. The Member is approximately 25 ft. thick.

35. The Onondaga Limestone is overlain by the Hamilton Group which includes, from older to younger, the Marcellus Formation, the Skaneateles Formation, the Ludlowville Formation, and the Moscow Formation. These formations are approximately 1,100 ft. thick in total and are comprised

mostly of silty shales and siltstones. Refer to Appendix B for further details. The formations of the Hamilton Group are generally exposed in ravines and gullies on the sides of the Onondaga Valley at elevations above the spillway.

Depositional History

36. In general, the sedimentary rocks of central New York, and at the site in particular, were deposited in a shallow sea that flooded most of New York State. The sea was bounded on the east by a landmass that occupied the Hudson Valley region. This landmass was uplifted into a region of high relief during the Late Ordovician and once again in the Middle and Late Devonian Periods. These are discussed further in Part III of this report. Between these periods of uplift the landmass was an area of low relief and may even have been flooded by shallow seas from time to time. The material comprising the shales, siltstones and sandstones, such as the Hamilton Group, was sediment being transported off this landmass by westward flowing streams. When this landmass was high, coarse-grained clastics such as sandstones reached central New York, when it was low, fine-grained clastics such as shale were deposited. When this area was very low or even flooded, the sea cleared allowing for the deposition of carbonates such as the Helderberg Group and the Onondaga Limestone.

37. The abundance and diversity of organic remains attests to normal marine conditions prevailing during deposition. The sea was probably warm, shallow, well lighted, and of normal salinity and oxygen content. Notable exceptions to this would be conditions represented by the Queenston and Grimsby Formations, Salina Group, and the black and dark shales of the Marcellus Formation.

Glaciation

38. During the Pleistocene Epoch, the study area was covered by glacial ice approximately 2 miles thick. The erosional effect of the ice was to deeply scour the Valley of Onondaga Creek into a deep U-shaped trough.

39. As the ice waned northward, its margin paused momentarily south of the site, depositing the Tully Moraine, near Tully, New York. With continued northward withdrawal of the ice, a thin veneer of ground moraine or glacial till was laid down on top of bedrock. The ice margin then paused along the northern edge of the Allegheny Plateau causing a proglacial lake to form south of the ice front and north of the Tully Moraine. Since the ice acted as a dam, the impounded waters were forced to flow eastward to the adjacent north-south valley (Butternut Creek Valley) across the sharp ridge between the two. Therefore, a dramatic series of west to east meltwater channels were carved across the highland ridge between the Onondaga Valley on the west and the Butternut Creek Valley on the east. As the ice margin slowly withdrew northward, successively lower channels were cut across the northerly sloping face of the Helderberg-Onondaga Escarpment marking the northern rim of the Allegheny Plateau. One of the more spectacular of these is the extinct waterfall of Clark Reservation State Park west of Jamesville. A large horseshoe shaped rim of a once high waterfall and plunge basin below (Green Lake) is all that remains of this meltwater channel.

40. As lower meltwater channels were carved, the lake in the Onondaga Valley was lowered until the ice finally pulled back from the rim of the Allegheny Plateau allowing the pent up waters to escape eastward to the Mohawk River draining the lake from the Onondaga Valley.

41. When the lake was present in the valley south of Syracuse, and as it gradually receded, it received meltwater from waters discharging eastward from Marcellus, through Pumpkin Hollow, to Cedervale (Figure 3). As the lake received meltwaters from the west, a dramatic series of glacial lake deltas were deposited in the vicinity of the site. These are the nearly flat terraces so pronounced in the immediate vicinity

of the dam, as well as at Gwills Corners, South Onondaga, Nichols Corners and Ironsides on the south side of the valley. They are known as hanging deltas and are shown on Figure 4. The west end of the dam is tied into one of the largest hanging deltas in the Onondaga Valley having a surface elevation of approximately 620 feet. The hanging deltas are excellent sources of coarse sand and gravel and are being actively quarried northwest of South Onondaga. Many smaller hanging deltas occur at lower elevations north of the site on both sides of the valley.

42. The surfaces of the hanging deltas correspond favorably with the threshold elevations of the various west to east meltwater channels, described above, leading eastward to Butternut Creek Valley. Therefore, the surface elevation of the lowering proglacial lake can be mapped and recognized.

43. At the dam, the soil profile, based on the geologic history, should reflect till above bedrock with lacustrine deposits above till in the middle of the valley but coarser deltaic deposits on the sides of the valley. Above this should be coarse deltaic sand and gravel as the deltas came closer to the site as a result of the lowering lake. Finally, overbank deposits and organic peat from Onondaga Creek should cap the section. A detailed discussion of the soil profile beneath the dam is presented in a later section of this report.

PART III: FIELD AND LABORATORY INVESTIGATIONS

Subsurface Exploration Program

44. Subsurface conditions in the vicinity of the dam were investigated prior to dam construction. Nine test borings, Nos. DH-1 through DH-4, DH-6, DH-8, DH-10, DH-11, and DH-16, were drilled approximately along the dam centerline as shown on Figure 5, Site and Subsurface Exploration Plan. A subsurface profile along the dam centerline, prepared by the Corps of Engineers, is shown on Figure C-1, Profile Along Dam Centerline, contained in Appendix C. Strata lines and geologic descriptions were added to Figure C-1 by H & A of New York.

45. The logs of the original test borings generally indicate compact foundation soils. However, near-surface soil and one deeper strata in DH-3, are noted to be semi-compact. The criteria used in the design phase explorations for the dam to evaluate the relative density of the soils is not known. Therefore, no specific quantitative data relative to soil density, such as blow counts, were available to make evaluations of the liquefaction susceptibility of the subsurface soils. Such data is required to assess liquefaction susceptibility and foundation stability under design earthquake loadings.

46. For these reasons, H & A of New York recommended that a limited subsurface exploration program be conducted to obtain relative density data in order to conduct liquefaction susceptibility analyses. It was recommended that three test borings be made from the dam crest, through the embankment and into the foundation soils. Standard penetration test results would be obtained and split spoon soil samples recovered for laboratory soil testing. By drilling the borings from the dam crest, data on soil type and density for the embankment materials could be obtained and added to the permanent record for the dam.

47. The three test borings, Nos. D82-101 through D82-103, were conducted during the period 28 July through 27 August 1982 by Parratt-Wolff, Inc. of East Syracuse, New York. These test borings were drilled generally in accordance with Corps of Engineers "Specifications for Subsurface Drilling Sampling and Testing". H & A of New York field

personnel monitored the test borings, described soil and rock samples, and prepared written test boring logs in accordance with standard Corps of Engineers procedures. Completed test boring logs are contained in Appendix D. As-drilled locations and ground surface elevations are shown on Figure 6. They were surveyed by Rowell and Associates, P.C., of North Syracuse, New York, using base information contained in "Plans for Onondaga Dam, Onondaga Creek, Syracuse, N.Y.", dated 1947. Elevations are referenced to Mean Sea Level Datum (MSL).

48. The test borings were drilled with truck-mounted Mobile Drill B-56, CME 55 and/or CME 75 drill rigs. Hollow stem augers with 3-3/4 in. inside diameters were used to drill through the dam embankment materials. Augers were fitted with a properly sized plug to prevent soils from entering the augers during drilling. Split spoon samples were obtained through the augers. After the augers were advanced to a depth of 60 to 78 ft., NW-size (3-3/16-in. I.D.) flush-joint casing was then set within the augers and the boring continued by driving the NW casing with a 350-lb. hammer. The casing was driven to the depth at which a sample was to be obtained and washed out using a fishtail bit. Split spoon samples were obtained below the bottom of the casing. When driving of the NW casing became difficult, BW size (2.75-in. I.D.) flush joint casing was set within the NW casing and the boring continued by driving the BW casing. In D82-103, driving of the BW casing became difficult at a depth of 150 ft. This hole was completed from this depth using an open hole filled with drilling fluid consisting of bentonite powder manufactured by N.L. Baroid. The test boring logs in Appendix A contain notes relative to the specific steps taken to advance each of the test borings.

49. Split spoon samples were taken in all test borings. When soils containing an appreciable gravel content were encountered in the embankment, a 2-7/8-in. I.D. by 3.5-in. O.D. split spoon was used. It was driven into the soil with a 350-lb. hammer free falling 18 inches. This large diameter spoon could only be used through the hollow stem augers. When sampling in all other instances, a standard 1-3/8-in. I.D. by 2.0-in. O.D. split spoon, was used. It was driven by a 140-lb. safety hammer free falling 30 inches. All split spoons were generally

driven 24 inches unless refusal was encountered. Refusal was defined as 50 blows of either hammer with less than 0.5 ft. of spoon penetration. Boulder or cobble obstructions were occasionally encountered and were removed by means of fishtail bits or tricone roller bits. The test boring logs contain notes relative to specific sampling procedures and obstruction removal procedures used for each of the test borings.

50. Sampling was conducted continuously in each of the test borings from the bottom of the embankment soils to a depth of 150 ft. or to the bottom of the hole as in D82-101. Continuous sampling was also conducted through the embankment materials in D82-102. At depths below 150 ft. and through the embankment materials in D82-101 and D82-103, sampling was conducted at 5-ft. intervals. Test boring D82-101 was terminated at a depth of 105.7 ft., D82-102 at 173.0 ft., and D82-103 at 184.8 ft.

51. When sampling below the subsurface water level, efforts were taken to generally maintain a positive head of water within the casing to prevent the soils below the casing from running into the casing. Running conditions were still encountered, however, within D82-102 at a depth of 63.0 ft.

52. Bedrock was encountered in D82-101 at a depth of 103.3 ft. During core drilling operations, it was discovered that the bottom portion of the casing had become bent when the bedrock was encountered, and only 1.9 ft. of rock was cored before a BX-size (1.375-in. I.D.) core barrel was sheared off within the hole. The core barrel was later retrieved and the hole terminated.

53. Upon completion of each test boring, the hole was filled with grout from the bottom of the hole to the top of the embankment. The casing was removed as the grout was placed. The grout consisted of a cement and bentonite mixture containing a small amount of sand.

Laboratory Testing Program

54. Laboratory soil tests were performed on selected samples obtained from the test borings in order to assist in classification of the samples. Natural water contents, Atterberg Limits, and grain size distribution tests were performed. Grain size distributions were obtained by means of mechanical sieve analyses and/or hydrometer analyses. All tests were conducted in accordance with accepted practice, generally as recommended by ASTM.

55. A total of five natural water contents and five Atterberg Limits determinations were made on fine-grained soils exhibiting some degree of plasticity (i.e., containing clay). Four of the samples tested were from the uppermost natural soil strata and one was from a cohesive layer within the lacustrine deposits found in D82-103. In addition, a hydrometer analysis was performed on each of these samples. A total of thirteen mechanical sieve analyses were performed. Four of the samples were from the embankment materials and nine from underlying natural soil strata. Representative samples were selected from most of the major soil strata.

56. Laboratory test results for all of the above tests are contained in Appendix E.

PART IV: SUBSURFACE CONDITIONS ALONG DAM CENTERLINE

57. The recent test borings were designated, from east to west, D82-101, D82-102, and D82-103 as shown on Figure 6. They encountered five significant soil deposits and bedrock. These were as follows:

- . Dam Embankment Materials
- . Fluvial Overbank Deposits
- . Deltaic Deposits
- . Lacustrine Deposits
- . Glacial Till
- . Bedrock

58. Bedrock was encountered in the eastern-most boring only, D82-101. Not all soil deposits were encountered in each test boring. A subsurface profile along the dam centerline developed from both the recent test borings and the original design test borings is shown on Figure 6. The following paragraphs describe the major soil deposits encountered in the recent borings, beginning with the embankment materials and progressing downward. Refer to Paragraphs 38 through 43 for a discussion of the geologic origin of the deposits.

Dam Embankment Materials

59. The dam embankment materials may generally be described as brown, silty coarse to fine SAND, little gravel, to coarse to fine sandy GRAVEL, little silt, with an occasional layer of medium to fine SAND, trace gravel, trace silt.

- a. Embankment materials were encountered to depths of 55.0, 65.5, and 65.5 ft. in borings D82-101, D82-102, and D82-103, respectively.
- b. Penetration resistance values were generally obtained with a 2-7/8 in. I.D. by 3.5-in. O.D. split spoon sampler driven by a 350-lb. hammer falling freely

18 inches. Blow counts for one foot of penetration ranged from 19 to refusal, defined as 50 blows with less than 0.5 ft. penetration. Generally, measured penetration resistances exceeded 50 blows/ft. A granular soil of this average penetration resistance can be described as very compact.

- c. Grain size distribution curves of typical embankment samples are contained in Appendix E as Test Nos. 1, 4, 5 and 9. For the samples tested, the embankment materials consisted of 34 to 56% gravel, 31 to 39% sand, and 13 to 28% silt.
- d. As a result of the above visual descriptions and laboratory test results, the dam embankment materials would be classified as GM, SM, or SP soils according to the Unified Soils Classification System (USCS).

Fluvial Overbank Deposits

60. The fluvial overbank deposits consist of gray and brown, fine sandy SILT, little clay, to silty CLAY, with an occasional layer of silty fine SAND. Small amounts of organic material are also present. They occur immediately below the embankment material, and are the original near-surface soils of the valley bottom.

- a. The deposit was 11.0, 6.5, and 5.0 ft. thick in borings D82-101, D82-102, and D82-103, respectively.
- b. Penetration resistance values were generally obtained with a 1-3/8-in. I.D. by 2.0-in. O.D. standard split spoon driven by a 140-lb. safety hammer falling freely 30 inches. Standard penetration resistance values of 1.0 ft. of penetration ranged from 17 to 34, with an average of 24. A cohesive soil of this average penetration resistance can be described as stiff; a granular soil as medium compact.

- d. As a result of the above visual descriptions and laboratory test results, the deltaic deposits would be classified as GM, GW, SM, and SP soils according to the USCS.

Lacustrine Deposits

62. Lacustrine sediments underly the deltaic deposits. These sediments consist of red-brown, silty fine SAND, trace coarse to medium sand and fine gravel, to silty CLAY, trace coarse to fine sand, with occasional layers of coarse to fine SAND, little gravel, little silt.

- a. The lacustrine deposit was encountered in D82-102 from a depth of 120.5 to 154.0 ft. and in D82-103 from 101.5 to 181.0 ft. It was not encountered in D82-101. The deposit was 33.5 and 79.5 ft. thick in D82-102 and D82-103, respectively.
- b. Samples were recovered with a standard split spoon sampler. Standard penetration resistance values ranged from 32 to 128 blows/ft. with an average of 77 blows/ft. A granular soil with this average penetration resistance would be described as hard.
- c. Grain size distributions of representative samples of coarser portions of this deposit (silty fine SAND) are contained in Appendix E as Test Nos. 8, 12, and 13. These soils contained 60 to 72% fine sand and 28 to 40% silt. Atterberg Limits No. A-5 and hydrometer analysis H-5 reported in Appendix E were performed on a sample of silty clay soil recovered in D82-103 from the uppermost portion of the deposit. This sample had a natural water content of 13.6%, a plastic limit of 12.9%, and a liquid limit of 21.7%, and consisted of 10% sand, 30% silt, and 60% clay.
- d. As a result of the above visual descriptions and laboratory test results, the lacustrine deposits would be classified as SM, SP, ML, and CL soils according to the USCS.

Glacial Till

63. Glacial till underlies the lacustrine deposit. Descriptions of recovered samples range from gray, gravelly, coarse to fine SAND, little silt, to brown, fine sandy SILT, little coarse to medium sand and gravel. Numerous cobbles and boulders were indicated during casing advance and sampling.

- a. This deposit was penetrated 19.0 and 3.8 ft. by D82-102 and D82-103, respectively. These borings terminated within this deposit. Glacial till was not encountered in D82-101.
- b. Standard penetration resistance values ranged from 71 blows/ft. to refusal. A granular soil of this penetration resistance can be described as very compact.
- c. No grain size distributions of this deposit were obtained.
- d. As a result of the above visual descriptions, the glacial till would be described as SM, GM and GW soils according to the USCS.

Bedrock

64. Bedrock was encountered in only one test boring; D82-101 at a depth of 103.3 ft. Attempts to core the rock met with limited success due to bent casing at the bottom of the hole. A run of 1.9 ft. was made using a BXM double tube core barrel with 1.6 ft. of rock core recovered. The recovered rock core has been described as a dark gray, slightly weathered, moderately hard, dense to finely crystalline, flat-lying LIMESTONE, with a few stylolites. It has been identified as the Manlius Limestone, Olney Member.

Subsurface Water

65. At the time of the dam construction, a series of piezometers and settlement gages were installed. The settlement gages were constructed such that subsurface water level readings could also be obtained. Readings have been obtained at semi-regular intervals since the dam was constructed. These readings indicate a subsurface water level of approximately EL 460± to 465±, closely corresponding to the original ground surface. During the recent subsurface exploration program, regular water level observations were made during drilling operations and the water level in adjacent settlement gages was measured. These levels, noted on the subsurface profile on Figure 6, also correspond closely with the original ground surface.

Subsurface Profile

66. The subsurface conditions along the centerline of the dam are quite variable. The previous subsurface explorations reveal that the soils of the east and west dam abutments are deltaic deposits. These deltaic deposits form the terraces rising above the Onondaga Creek flood plain. However, the recent test borings along with the previous test borings show that, within the limits of the original flood plain beneath the dam, a deposit of fluvial overbank sediments overlie deltatic deposits, which in turn overlie lacustrine deposits. Glacial till and bedrock lie below the lacustrine deposits. The depths and thicknesses of these deposits vary from east to west.

- a. The silt and clay overbank deposits were encountered between test borings DH-6 and DH-2. The overbank sediments are approximately 5 to 13 ft. thick at test boring locations, within the flood plain, and pinch out to the east and west at the valley sides.
- b. Sand and gravel deltaic deposits underlie the overbank deposits in the central portion of the

dam and make up the east and west abutments. At the test boring locations, the deltaic sediments beneath the dam appear to vary in thickness from approximately 30 to 55 ft. This deposit thins to the east where the bedrock surface rises. The deltaic deposits of the west abutment consist of two units; a coarse-grained upper portion and a fine-grained lower portion. The deltaic deposits of the east abutment are predominantly granular and overlie bedrock.

- c. Underlying the deltaic deposit, along approximately the western one-half of the dam centerline, is a lacustrine deposit consisting of silt and fine sand. Based on test boring results, it appears the deposit thickens to 80 ft. and extends beneath the terrace at the west abutment. Towards the east it pinches out due to the rising bedrock surface.
- d. Underlying the lacustrine deposit, is a deposit of glacial till, a homogeneous mixture of silt, sand, and gravel. The top of the glacial till deposit slopes to the west as shown by the test borings and is probably closely underlain by bedrock.
- e. Test borings along approximately the eastern one-third of the dam and the east abutment encountered limestone bedrock immediately beneath the deltaic deposit. The bedrock surface appears to drop off sharply to the west, as rock was not encountered west of boring DH-2. It is H & A of New York's opinion that the rock cored in the bottom of DH-3 is a boulder in the till.

PART V: TECTONIC HISTORY

Orogenies

67. Orogenic events, or mountain building episodes, have not occurred *per se* in central New York. However, orogenies have occurred within and outside the state boundaries that have effected the depositional and structural history of the region.

The orogenic events are as follows:

- a. Taconic Orogeny - This is the earliest mountain building episode to strike the state and took place in the Late Ordovician and Early Silurian Periods (Table I). It resulted from compressive forces that became established as ancestral Africa and Europe began to move closer to ancestral North America. These compressive forces folded and thrust faulted the area of eastern New York and western New England forming the Taconic Mountains of New York, Berkshire Hills of Massachusetts, and the Green Mountains of Vermont. Streams flowing westward of this mountainous highland (sometimes called Vermontia) deposited a westward migrating delta or clastic wedge across the state. It is known as the Queenston Delta and the Queenston and Grimsby Formation represent the terrestrial uppermost portion of this delta.
- b. Acadian Orogeny - The Acadian Orogeny took place during Middle and Late Devonian times immediately after the deposition of the Onondaga Limestone. It continued into the succeeding Mississippian Period as well. The Acadian Orogeny struck the same general area as the earlier Taconic Orogeny. However, from the amount of clastic sediment shed from the highland area and other data, this orogeny was larger than the Taconic. It came about as a result of the collision of ancestral Africa and Europe with northeastern North America. This orogeny was the

climax of the Appalachian Mountains uplift in the north. Another delta of clastic wedge was shed westward off the Acadian Landmass known as the Catskill Delta. It begins with the clastic sedimentation of the Hamilton Group and continues into the Upper Devonian (Table II). By the end of the Devonian Period this delta had prograded westward across the entire state or nearly so.

- c. Appalachian Orogeny - This orogeny struck mostly the southern Appalachian chain as ancestral Africa rotated to collide with southeastern North America. This took place primarily during the Late Paleozoic Era, the Pennsylvanian and Permian Periods. The effect on central New York from this orogeny was to impart to the strata a dip of 40 to 50 feet per mile to the south. In addition, the joint system was formed at this time as well as the minor faults and folds discussed below.
- d. Mesozoic Events - After the Appalachian Orogeny the present Atlantic began to open as Europe and Africa rifted from North America and drifted gradually eastward. Tensional forces resulting from this breakup formed the downfaulted Triassic Lowland Province of southeastern New York (Figure 2). In addition, upward migrating magmas formed the Palisades Sill of the Hudson and also may have been responsible for the many small dikes that occur throughout central New York. These dikes are shown on Figure 3, and discussed below.

Structural Deformation

68. Superimposed on the regional dip approximately $1/2^{\circ}$ to the south are local folds, consisting of low amplitude anticlines and synclines, and faults of low displacement.

Folds

69. A broad anticline and syncline is exposed in the Chittenango Member of the Marcellus Formation on the east side of N.Y. Route 173 just south of its intersection with Slate Hill Road, south of Marcellus.

70. Along the east side of Interstate 81, south of the junction of Interstate 481, there are a series of gentle anticlines and synclines. The height of these folds is approximately 30 feet. They are a few hundred feet wide and expose the Manlius Formation (Olney, Elmwood, Clark Reservations and Jamesville Members).

71. A gentle anticline is exposed in the spillway cut at the dam. The Manlius-Onondaga contact rises to a maximum of 12 feet above the floor of the valley from the spillway northward, then descends further northward such that it is near creek level at the northern extremity of the exposure.

72. A monocline in the Onondaga Limestone is exposed on the south side of the Seneca Turnpike (NY Route 173) at Fillmore Corner, which is at the intersection of Sweet Road, about 2.5 miles east of Jamesville. The Onondaga is horizontal on each side of the monocline but is bent nearly vertical within the structure. The Onondaga is some 23 feet lower on the downside of the monocline. Outside of the Onondaga Valley region, a broad anticline is exposed along Cayuga Lake north of Ithaca, known as the Portland Point anticline.

73. The folds in the Syracuse and Central New York region are related either to mild compression as a result of the Appalachian Orogeny or to removal of salt and gypsum in the Salina and Bertie Groups due to migrating ground water. Thus some of the smaller folds may be subsidence structures and not uplift structures formed by compression.

Faults

74. A number of small faults occur in the Syracuse - Central New York region as revealed by a search of the literature augmented by field work. All of these are small in lateral extent and displacement or offset. Further, they strike approximately $N70^{\circ}W$, dip mainly southward, are mostly thrust faults that form due to compression, and therefore, are probably Late Paleozoic Appalachian Orogeny structures (approximately 220 million years old). Overlying glacial deposits are not offset indicating that no motion has taken place on these faults at least since the last retreat of the ice, approximately 14,000 years ago.

75. A small thrust fault of 3 feet vertical displacement occurs in the Old Malley quarries south of Marcellus and a few hundred yards north of the anticline described above in the Chittenango Shale Member. The fault cuts the Onondaga Limestone.

76. A thrust fault, with a throw of about 5 feet, in the Onondaga Limestone and extending into the overlying Union Springs Member can be seen in the drainage ditch on the east side of U.S. Route 11, south of the Nedrow interchange to Interstate 81 at the intersection of Kennedy-Camping Road. The Union Springs Shale Member is highly contorted and tightly folded, apparently absorbing the fault motion. This fault is the closest thrust fault to the dam, a distance of approximately 2.5 miles, and cannot be traced beyond this one exposure.

77. A thrust fault in the Jamesville Quarry on the east side of the Butternut Creek Valley, north of Jamesville, has a vertical displacement of 16.5 feet in the Onondaga Limestone. This structure probably grades eastward into the monocline described above, exposed at Fillmore Corner.

78. One of the largest faults in the Syracuse area formerly was exposed in the old Russell's Quarry, now buried near the junction of N.Y. 173 and LaFayette Road in the southeast corner of the Syracuse West quadrangle. The fault plane dips south 20° and displaces the Manlius and Onondaga limestones 42 feet. South of Jamesville, on the east side of the Butternut Creek Valley, in the Onondaga County Penitentiary Quarry, a thrust fault of about 31 feet may be the same fault as the one

formerly exposed in Russell's Quarry. If so, the fault is approximately 3.5 miles long parallel to strike.

79. Four small thrust faults with throws of 2 to 4 feet cut the Bertie Group in the Fiddlers Green Gorge at Jamesville. Three thrust faults of comparable strike, dip, and displacement to those described above have been discovered on the Manlius Quadrangle to the east.

80. A series of small normal faults, down thrown to the north, can be seen along the southwest side of the Tully Valley in Lords Hill Ravine and Cass Hill Ravine, south (upstream) of U.S. Route 20, having a total displacement of approximately 45 feet. The fault planes strike $N60^{\circ}W$, and are nearly vertical. The Staghorn Point and Joshua Submembers are cut by the faults.

81. The unusual thickness of the Pompey Member in Bare Mountain Ravine may also be due to normal faulting, but the lack of outstanding key beds makes this hypothesis difficult to test.

Joints

82. The joint system in the Syracuse region contains two main sets, one nearly north-south and the other nearly east-west. Most of the joints in these sets are vertical or nearly so. Two minor sets that strike northeast and northwest are also present.

83. Inclined joints that strike about $N70^{\circ}$ to $75^{\circ}W$ and dip 30° to 80° either north or south are locally developed in the limestones in the Syracuse area. Some of the inclined joints are continuous for many tens of feet while others are limited to a single bed or group of beds and end against a shaley parting. They are well developed in parts of the Manlius Formation such as the Clark Reservation Limestone Member and the Onondaga Limestone, especially the Nedrow Member.

Dikes

84. In central New York, small dikes from a few inches to a few feet wide occupy the North-South joint set. They are composed of kimberlite

and alnoite, a high temperature, ultra-basic, igneous rock, high in iron and magnesium silicates and low in aluminum silicates such as potash feldspars.

85. One dike occurs in North Syracuse near Schiller Park, one at the DeWitt Reservoir 3 miles east of Syracuse, and another near Otisco Lake, 15 miles southwest of Syracuse. A relatively large number are exposed in Ithaca and on both sides of Cayuga Lake north of Ithaca to Taughannock Falls State Park.

PART VI: EARTHQUAKE ACTIVITY

Historic Earthquakes

86. More than 330 earthquakes with a maximum Modified Mercalli Intensity (I_0) greater than II are known to have occurred in New York State between 1720 and 1980 (Mitronovas, 1981). Figure 7 is a seismicity map for the northeast U.S. and adjacent Canada, showing all known historical and recorded events of maximum intensity greater than or equal to III for the time period 1534 to 1977. Figure 7A is a seismic zone map for the contiguous United States and Puerto Rico with the corresponding location of the dam site. An abbreviated form of the Modified Mercalli scale is shown on Figure 8, along with approximate relationships between intensity, Richter magnitude, and peak horizontal ground acceleration.

87. All earthquakes with a maximum intensity greater than or equal to IV that have occurred within 200 kilometers of the Onondaga site are listed in Table IV in order of decreasing epicentral intensity.

Geographic Distribution of Earthquakes

88. As discussed by Mitronovas (1981) and shown on Figure 9, New York State can be subdivided into three areas of relatively high seismic activity, separated by a large area of very low or no activity in the center of the State (which includes the Onondaga Dam site). These regions were designated A, B, C, and D by Mitronovas. Over 90 percent of all seismic events, and all events of maximum intensity greater than V, fall into regions A, B, and C, which have a combined area of less than 60 percent of that of the State. Table V gives a chronological list of earthquakes within New York State of maximum intensity VI or greater. The following discussions of the seismicity of the four regions of New York State are taken from Mitronovas (1981).

89. Region A is an area of moderate seismic activity. It contains the Ramapo fault system, which has been associated with recent seismic activity. One of the earliest documented large earthquakes in New York (I_0 = VII) occurred in Region A near New York City on 18 or 19 December 1737.

90. The Adirondack area, region B, is the most seismically active region of the state. It is part of a larger seismic area which trends northwest-southeast and extends well into Canada. There is evidence that the Adirondacks are undergoing doming, or upward movement (Isachsen, 1975). The doming may be linked to the seismic activity of the area, but this is not known for certain. The largest known earthquake within New York State ($I_0 = VIII$) occurred in this region near Massena, New York on 5 September 1944.

91. The number of known earthquakes within region C is smaller than in either of regions A or B. However, one of the two largest earthquakes in the state, near Attica, occurred in region C on 12 August 1928. The general location of this event suggests that it was associated with the southern end of the Clarendon-Linden fault system. Past and present seismicity of region C is not restricted to this fault system alone, however.

92. The central part of the state, region D, has very little seismic activity, both at present and historically. This large aseismic area separates the seismically active areas of the state into three distinct regions.

93. The available data (Mitronovas, 1981) indicates secular variation, or periodicity over time, in seismic activity in New York since 1920. Periods of greater activity, in terms of total number of events per unit of time, between 1760 and 1790, 1830 and 1880, and 1910 through the present, are separated by periods of less activity between 1790 and 1830 and 1880 and 1910. An increase since 1910 is consistent with the improvement in detection capability. Chiburis (1981) has indicated that the Adirondack region has exhibited a decreased seismic activity since 1925. The reasons for time-dependent seismicity in the northeast U.S. are not known.

PART VII: EVALUATION FOR ACTIVE FAULTS

94. Earlier sections of this report have established that mapped faults in the site vicinity are ancient ones which date back to the Appalachian Orogeny of Late Paleozoic time (over 220 million years ago). Glacial overburden and urban construction have made it all but impossible to trace faults for any great distances. The evidence indicates that all the faults are inactive at the present time and are small local occurrences of limited extent.

Association of Earthquakes with Tectonism and Faults

95. The earthquake distribution in New York State bears little or no proven relationship to known active or inactive faults. In region A, there is some activity that corresponds to the Ramapo fault system, but not all activity is confined to this system. In region B, the relationship to known faults or other tectonic features is not clear. The correlation between epicenters and known surface faults or other brittle structures still remains to be determined. In region C, the past and present seismicity is not restricted to the Clarendon-Lindon fault system alone. Indeed, although the seismicity extends both east and west of the fault system, it does not extend northward along the fault system, which is known to extend into Lake Ontario. The relationship of most of the epicenters in region C to tectonic structures is, as in region B, not yet known.

96. The Attica seismic region (region C) has produced several events for which some possible fault types and orientations have been obtained, based on seismological data. Hermann (1978) determined similar focal mechanisms for both the 1 January 1966 and 13 June 1967 events. A plane striking NNE parallels the trend of the Clarendon-Linden fault structure. If this is indeed the fault plane, then the motion along this plane has approximately equal components of right-lateral strike slip and reverse

faulting. Composite fault-plane solutions obtained by Fletcher and Sykes (1977) for Attica events between 1972 and 1975 have one plane also oriented NNE, but their solution indicates predominantly reverse faulting.

Lineations

97. Lineations or linear features, are those that are found in tonal changes in satellite imagery, and in the alignment of rivers, terrace boundaries, etc. They may be the result of a multitude of causes and may be unrelated to faults.

98. The Brittle Structures Map of New York State, compiled by Isachsen & McKendree (1977) was reviewed to note locations of any lineations in the site vicinity. Several lineations were noted. An attempt was made to check these in the field but nothing could be discerned. They appear to be unrelated to faults but may be due to jointing or glacial erosion and deposition. None of the faults or linears show any evidence of activity in the area of the project.

PART VIII: INTENSITY OF EARTHQUAKE SHAKING AT ONONDAGA DAM

Earthquake Intensities and Attenuation

99. All earthquakes with a maximum intensity greater than or equal to IV that have occurred within 200 kilometers (km) of the site are shown in Table IV. Because larger earthquakes at distances greater than 200 km may produce significant site intensities, all 2705 events in the Chiburis (1981) earthquake catalog have been attenuated to the Onondaga site. The attenuation relation used was that of Cornell and Merz (1975):

$$I = I_0 + 3.211 - 1.3 \ln (\text{distance in km})$$

100. Table VI shows the specific events which produce site intensities greater than or equal to IV when attenuated to the site.

101. As the data clearly show, the maximum shaking intensity experienced at the Onondaga Dam site has been IV to V. (Intensities noted in Table VI are in arabic numerals because of fractional values). The peak horizontal ground acceleration was therefore in the range of 1 to 3 percent of the acceleration due to gravity (0.01g to 0.03g). The event that produced the highest intensity at the site was the 1663 La Malbaie earthquake ($I_0 = X$) at a distance of about 700 km from the site. Because of the early occurrence at this event, the assigned intensity of X, based on historical accounts (written or otherwise) is at best only an estimate. Of more concern would be the Attica and Massena earthquakes, recorded in 1929 and 1944, at distances of 180 km and 250 km from the site, respectively. The magnitude of the Massena event was reported to be 5.9, whereas the magnitude of the Attica event was estimated from historical records to range from 5.5 to 5.9 (Chiburis, 1981). However, the attenuated intensities at the dam are still only on the order of IV. A possible faulting mechanism for the Attica region was discussed earlier in this report (para. 96).

102. Iseoseismals from the 1 March 1925 intensity IX event centered in La Malbaie, Quebec are shown in Figure 10. This event resulted in

intensity IV shaking at the site, and would have caused rattling of dishes and windows and disturbance of doors and cars near the site. The reduction in five levels of intensity from La Malbaie to central New York for the 1925 event is consistent with the attenuation of the 1663 La Malbaie earthquake discussed in the previous paragraph.

103. It is concluded that earthquakes with epicentral intensity greater than IV will not have epicenters near the Onondaga site. The nearest regions of concern are the Attica and Massena regions, which may produce intensities of IV to V at the site. The focal mechanism of the Attica region is most probably reverse or thrust faulting with north-easterly trending fault planes.

Frequency of Earthquake Occurrence and Seismic Risk

104. A seismic risk analysis was performed to aid in the selection of a level of earthquake shaking for use in assessing the seismic stability of Onondaga dam. The analysis was used to estimate the probability of exceeding various levels of earthquake shaking within a specified time period. These calculated probabilities allow for selection of an "analysis" earthquake that is consistent in terms of seismic risk with other similar structures.

105. The procedure used to calculate the probability of exceedence of specific levels of earthquake shaking at the site was to first divide the northeastern United States and adjacent areas into more than 50 zones or potential sources of seismic activity. Each zone has its own assigned level of seismic activity based on historical seismicity. The zones are quadrilateral in shape and are based on the regionalization given in Chiburis (1981). It is assumed that the locations of potential earthquake point sources (epicenters) are randomly distributed with each zone. In addition to the specific zones, a low-level "background" seismicity was assumed, randomly distributed over the entire northeast area. This allows for the possibility of earthquakes occurring at any location, not just in zones where historic seismicity is relatively concentrated.

106. The probability of occurrence of intensity $i \geq I_1$ shaking at the site was calculated based on the "Total Probability Theorem" as

$$P(i \geq I_1) = \sum_{\substack{\text{all} \\ \text{source} \\ \text{zones}}} \sum_m P(i \geq I_1 | m) P(m)$$

in which:

$P(i \geq I_1)$ denotes the probability of the occurrence of intensity $i \geq I_1$ at the site;
 $P(m)$ denotes the probability of occurrence of an earthquake of magnitude m or greater in a source zone;
 $P(i \geq I_1 | m)$ denotes the probability of occurrence of intensity $i \geq I_1$ at site given the occurrence of a magnitude m event in the source zone.

107. The probability calculations were performed on a computer using a program developed by McGuire (1977). The procedures used to calculate the needed parameters for each zone are described in the following two paragraphs.

108. The probability of occurrence of an earthquake of magnitude m or greater within a source zone, $P(m)$, is determined from the assigned recurrence relationship for each zone. The recurrence relationship is of the form:

$$\log N = A - Bm$$

in which N is the average number of events per year of magnitude m or greater for each specific source zone, and A and B are constants for the zone. The probability of occurrence of an earthquake of magnitude m or greater is determined by numerical integration of the above recurrence relationship for each zone. The "B" parameter was assumed to be equal for all zones except La Malbaie, which exhibits much more seismic activity than other areas in the northeast. The "A" parameter was determined from historical seismic activity for each zone.

109. The probability of intensity i shaking at the site given a magnitude m event in a source zone, $P(i \geq I_1 | m)$, is related to the attenuation relationship for the site area. Attenuation is assumed to follow a normal probability distribution. The mean value of the distribution was based on Cornell and Merz (1975) as presented in paragraph 99. Chiburis (1979) compared the Cornell and Merz relationship to observed attenuation for six northeastern events, as shown on Figure 11. The relationship compares well with the observed data in the near and far fields, although it is not as good in the intermediate range. To account for the variability in the observed attenuation in the analysis, a standard deviation of 0.4 was assumed. This is more conservative than the 0.2 value used by Cornell and Merz.

110. The probability of exceedence results calculated with the computer program are based on a one-year time period. The probability of exceedence of a specified intensity I_1 in T years can be calculated from the following:

$$P(i \geq I_1) = 1 - e^{-\lambda T}$$

in which λ is the mean number of events per year of intensity I_1 . For the levels of seismic risk considered herein, λ is also equal to the probability of exceedence of intensity I_1 in one year.

111. Figure 12 depicts the results of the analysis, that is, the probability that specified earthquake intensities and corresponding peak horizontal ground accelerations will be exceeded at the site in a 50-year period. Intensity was converted to peak horizontal ground acceleration in rock using the relationship shown on Figure 8. The following is a brief summary of selected results:

PEAK HORIZONTAL GROUND ACCELERATION IN ROCK	INTENSITY	PROBABILITY OF EXCEEDENCE IN 50 YEARS
0.02g	V	7.0%
0.05g	VI	1.0%
0.10g	VII	0.8%

On the average, intensity V, VI and VII shaking would be expected to recur at the site every 650, 4,400 and 65,000 years, respectively.

PART IX: ASSESSMENT OF EARTHQUAKE EFFECTS ON ONONDAGA DAM

112. Based on the results of the seismic risk analysis and current engineering practice, the peak horizontal ground accelerations in rock recommended for use in assessing the seismic stability of the Onondaga Dam is 0.05g. This acceleration corresponds to an intensity of shaking of VI, which is one intensity level higher than the site is known to have experienced historically. The probability that this level of shaking would be exceeded in a 50-year period is 1.0 percent based on the analysis performed.

113. The duration of strong motion in an earthquake is often defined as the time interval between the times that a 0.05g acceleration is first and last achieved. This definition would result in a duration of zero for Onondaga Dam. Nuttli (1979) has defined duration as the time interval between the times that one-half the "sustained maximum" (third highest acceleration peak) acceleration is first and last achieved. This definition leads to longer durations for small earthquakes. Using this definition of duration and considering that the epicenter of the causative earthquake is likely to be more than 100 km from the site, the recommended duration of shaking is approximately 30 seconds.

114. The recommended level of earthquake shaking could only cause embankment or spillway instability, if a marginal static stability condition exists for either the embankment dam or concrete spillway. Based on the calculations presented in the U.S. Engineer Office Design Analysis for the dam (Ref. 17), the static stability of the embankment and spillway appears adequate. Significant settlement or liquefaction of foundation soils could only occur if the foundation soils are cohesionless and very loose to loose in relative density. In addition, for liquefaction to occur, the foundation soils must be saturated. The logs of the design phase (1945) borings indicate that foundation soils at the dam are predominantly compact to very compact, saturated cohesionless soils, but the criteria used to evaluate the relative density

of the soils is not known. No penetration test results or quantitative data of any kind were available. The recent test borings were undertaken as part of this study in order to provide such data. Engineering studies relative to the seismic stability of the dam were then performed using the data obtained from the borings.

115. The subsurface conditions along the dam centerline are shown on Figure 6 and have been summarized in paragraphs 57 to 66. As noted in paragraph 65, subsurface water levels are generally between El. 460 to 465.

116. A plot of Standard Penetration Resistance (N-value) versus sample depth below the top of the embankment for the recent test borings is provided in Figure 13. N-values obtained by means of standard methods, 1-3/8 in. I.D. 2.0-in. O.D. split spoon driven by a 140-lb. hammer free falling 30 inches, are shown on the plot without correction. N-values obtained by means of the large split spoon, 2.875-in. I.D. 3.5-in. O.D. driven by a 350-lb. hammer free falling 18 inches, have been converted to standard values for use on the plot. The conversion has been made on the basis of sampler-hammer ratios which take into account sampler size, hammer weight, and drop height by the following equations:

$$R_S = (D_O^3 - D_i^3)/WH \text{ (144) ft.}^2/\text{lb. for granular soils}$$

$$R_S = (D_O^2 - D_i^2)/WH \text{ (12) ft./lb. for cohesive soils}$$

where;

R_S = Sampler - Hammer Ratio

D_O = Outside Diameter of Sampler (in.)

D_i = Inside Diameter of Sampler Shoe (in.)

W = Weight of Hammer (lb.)

H = Height of Drop (in.)

Calculations of the sampler-hammer ratios and the conversion charts used are contained in Appendix F.

117. In addition, projected N-values have been used for samples which encountered refusal before reaching the required one foot of

penetration. In these cases the recorded penetration resistance values were projected by direct proportion to the one foot standard for use on the plot.

118. The liquefaction potential of the saturated cohesionless foundation soils was analyzed in accordance with the methodology of Seed and Idriss (1981). The calculations performed for the assessment of liquefaction potential are provided in Appendix F. A summary of the analytical procedure is as follows:

1. Calculate the effective ($\bar{\sigma}_{v_0}$) and total (σ_{v_0}) overburden pressures below the crest and toe of the dam.
2. Calculate the seismic shear stresses (τ) imposed by the earthquake on the foundation soils and obtain the seismic shear stress ratio, $\tau / \bar{\sigma}_{v_0}$, as a function of depth below the toe and crest of dam.
3. Correct Standard Penetration Resistance values to N_1 values, the N -value at an effective overburden pressure of one ton per sq. ft. (tsf.)
4. Plot $\tau / \bar{\sigma}_{v_0}$ versus N_1 and compare to data obtained from previous earthquakes for known cases of liquefaction and nonliquefaction.

119. To determine the seismic shear stress as a function of depth, it is necessary to estimate the amplification of the bedrock earthquake motions as caused by the presence of the overburden soils. The amplification by the dense soils at the site was estimated by taking the ratio of the response spectra peak for "stiff" soil conditions to the peak for "rock", for the spectra shown on Figure 14. The ratio of the "stiff" soil peak to the "rock" peak was 1.16. An amplification factor of 1.2 was assumed in the analysis, resulting in a peak horizontal ground acceleration of 0.06g at the natural ground surface.

120. The results of the liquefaction analysis are shown on Figures 15 and 15A, which shows the plot of $\tau / \bar{\sigma}_{v_0}$ versus N_1 for the dam foundation soils. The figures show that even the lowest N_1 values obygained at the site lie

to the "safe" side of the boundary line separating regions of observed liquefaction from nonliquefaction. Therefore, liquefaction of the foundation soils is not believed to be possible under the recommended levels of earthquake shaking.

121. It should be noted that the N_1 range shown on Figures 15 and 15A is the range of the lowest N_1 values encountered at the site. The average N_1 for the foundation soils would be significantly greater than the highest end of the range shown.

122. The embankment soils were disclosed by the borings to be medium compact to very compact, as noted earlier. In addition, these soils were found to be partially saturated. The results of the foregoing analyses provide a basis for concluding that the embankment soils would not be liquefaction-susceptible for the recommended level of shaking if they were to become saturated due to an impoundment.

PART X: CONCLUSIONS

123. The Onondaga Dam is located in a region of exposed gentle, southerly sloping Lower and Middle Devonian limestones and shales of the Helderberg Group, Onondaga Limestone and Hamilton Group. Superimposed on the dip are some minor, small faults and folds of Late Paleozoic age. Therefore, the dam is situated in an area that is structurally simple and tectonically stable.

124. The region has been glaciated, and the valley in which the dam is located is a U-shaped glacial trough. Upon retreat of the ice, large, hanging delta terraces of sand and gravel were deposited in the valley in a gradually receding proglacial lake that escaped eastward through meltwater channels.

125. The dam is located in an area that historically can be described as nearly aseismic. The intensity of ground shaking recommended for use in assessing the seismic stability of the dam and its foundations is VI, with a corresponding peak horizontal ground acceleration of 0.05g in rock and 0.06g at the surface of natural ground where the dam overlies soil deposits. The average length of time between occurrences of this degree of shaking at the site is 4,400 years, based on probability analyses. The calculated probability that the above intensity will be exceeded at the site in a 50-year period is 1.0 percent. Possible sources of earthquakes which might produce this degree of shaking are the Attica and Massena, New York areas and the La Malbaie, Quebec area.


126. To obtain qualitative and quantitative data for use in the assessment of the stability of the dam under earthquake shaking, three test borings were drilled from the crest of the embankment into the foundation soils as a part of this study.

127. The approximately 65-ft. high embankment consists of medium compact to very compact sands and gravels that were not saturated at the time of this study. Immediately below the embankment is a relatively thin layer of saturated, stiff to hard, clayey silt and silty clay.

Below this layer, foundation soils consist primarily of saturated, medium compact to very compact sand with varying amounts of gravel and silt, to a depth of approximately 35 to 65 ft. below the base of the dam. Below this granular deposit is a medium compact to very compact fine grained deposit of variable thickness overlying glacial till. On the east side, bedrock is relatively close to the bottom of the embankment.

128. The static stability of the embankment and foundation soils is adequate based on the type and relative density of the soils present and the geometry of the dam. Likewise, the stability of the concrete spillway, which bears on rock, also appears to be adequate.

129. The liquefaction susceptibility of the soils at the site was analyzed for an earthquake producing 0.06g peak acceleration at the surface of natural ground. The embankment and foundation soils are not considered to be susceptible to liquefaction for this level of earthquake shaking. Minor seismically-induced settlement of the embankment and subsoils may occur, but the settlement will not be detrimental to the performance of the structure.

130. As a result of the analyses conducted, it is the opinion of H & A of New York that a more in-depth analysis of the stability of the dam due to liquefaction or other earthquake effects is not warranted. 

REFERENCES

1. Applied Technology Council, "Tentative Provisions for the Development of Seismic Regulations for Buildings," ATC Publication ATC 3-06, 1978.
2. Chiburis, E.F., "Seismicity, Recurrence Rates, and Regionalization of the Northeastern United States and Adjacent Southeastern Canada," U.S. Nuclear Regulatory Commission, NUREG/cr-2309 RA, R6, 1981.
3. Chiburis, E.F., Ahner, R.O., and Graham, T., "Seismicity of the Northeastern United States," Northeastern U.S. Seismic Network Bulletins, Weston Observatory, Boston College, 1979.
4. Cornell, A. and Merz, H., "Seismic Risk Analysis of Boston," Jour. Struc. Div. Proc. Am. Soc. Civil Engineers, V. 101, No. 10, 1975, pp. 2027-2043.
5. Fletcher, D.B. and Sykes, L.R., "Earthquakes Related to Hydraulic Mining and Natural Seismic Activity in Western New York State," J. Geophys. Res., V. 82, 1977, pp. 3767-3780.
6. Hermann, R.B., "A Seismological Study of Two Attica, New York, Earthquakes," Bull. Seism. Soc. Am., V. 68, No. 3, 1978, pp. 641-652.
7. Hutchinson, D.R., Pomeroy, P.W., Wold, R.J., and Hallis, H.C., "Geophysical Investigations Concerning the Continuation of the Clarendon Linden Fault Across Lake Ontario," Geology, V. 7, No. 4, 1979, pp. 206-210.
8. Isachsen, Y.W., "Possible Evidence For Contemporary Doming of the Adirondack Mountains, New York, and Suggested Implications For Regional Tectonics and Seismicity," Tectonophysics, V. 29, 1975, pp. 169-181.
9. Isachsen, Y.W., and McKendree, W.G., "Preliminary Brittle Structures Map of New York," New York State Museum Map and Chart Series No. 31B, 1977.
10. Krinitzky, E.L., and Patrick, D.M., "Earthquake Investigations at the Dickey-Lincoln School Damsites, Maine," U.S. Army Engineer Division, New England, Miscellaneous Paper S-77-2, Final Report, 1977.
11. McGuire, R.K., "FORTRAN Computer Program For Seismic Risk Analysis," U.S. Dept. of Interior, U.S. Geological Survey, Open File Report 76-77, 1977.

REFERENCES (continued)

12. Mitronovas, W., "Earthquake Statistics in New York State." In Press, 1981.
13. Nuttli, O.W., "The Relation of Sustained Maximum Ground Acceleration and Velocity of Earthquake Intensity and Magnitude, "State of the Art for Assessing Earthquake Hazards in the United States," Report 16, Misc. Paper S-73-1, U.S. Army Engineers W.E.S., Vicksburg, Miss., 1979.
14. Oliver, W.A., Jr., "Middle Devonian Coral Beds of Central New York," Amer. Jour. Sci., V. 249, No. 10, 1951, pp. 705-728.
15. Seed, H.B. and Indriss, I.M., "Evaluation of Liquefaction Potential of Sand Deposits Based on Observations of Performance in Previous Earthquakes," In Situ Testing to Evaluate Liquefaction Susceptibility, ASCE Preprint 81-544, ASCE National Convention, St. Louis, MO, 1981.
16. U.S. Atomic Energy Commission, "Nuclear Reactors and Earthquakes," TID-7024, Chapter 1, 1963, pp. 13-14.
17. U.S. Engineer Office, Syracuse District, "Design Analysis for Onondaga Reservoir, Onondaga Creek, New York," Oswego River Watershed Flood Control, 1945.
18. U.S. Engineer Office, Syracuse District, "Plans for Onondaga Dam, Onondaga Creek, Syracuse, New York," Oswego River Watershed Flood Control Project, 1947.

ONONDAGA DAM, NEW YORK

TABLES

U. S. Army Engineer District, Buffalo
1776 Niagara Street
Buffalo, New York 14207

TABLE I
GEOLOGIC TIME SCALE

			TIME BEFORE PRESENT (MILLIONS OF YEARS)	
ERA	PERIOD	EPOCH	BEGINNING	DURATION
CENOZOIC	Quarternary	Holocene (recent)	0.01	
		Pleistocene	3 ⁺	2.99
	Tertiary	Pliocene	5	2 ⁺
		Miocene	25	20
		Oligocene	34	9
		Eocene	49	15
		Paleocene	64	15
MESOZOIC	Cretaceous		130	66
	Jurassic		180	50
	Triassic		220	40
PALEOZOIC	Permian		270	50
	Pennsylvanian		325	55
	Mississippian		355	30
	Devonian		410	55
	Silurian		430	20
	Ordovician		500	70
	Cambrian		600	100
PRECAMBRIAN				
FORMATION OF THE EARTH			4,600	

NOTE: Table derived from various geologic sources.

TABLE I

TABLE II
GENERALIZED STRATIGRAPHIC COLUMN - CENTRAL NEW YORK

GENERALIZED STRATIGRAPHIC COLUMN CENTRAL NEW YORK					APPROXIMATE THICKNESS IN FEET	
	SYSTEM	SERIES	GROUP	FORMATION		
DEVONIAN	Upper Devonian	Chautauquan	Conewango Conneaut Canadaway	(Many formations recognized in each group)	3000	
		Senecan	West Falls Sonyea Genesee		2600 670 to 800 900 to 1450	
				Tully Ls.	15 to 100	
	Middle Devonian	Erian	Hamilton	Moscow Ludlowville Skaneateles Marcellus	160 to 240 300 300 to 500 100 to 500	
				Onondaga Ls.	70 to 100	
	Lower Devonian	Ulsterian	Tristates	Oriskany Ss.	0 to 10	
			Helderberg	Manlius Ls. Chrysler Dol.	0 to 200 15	
	SILURIAN	Upper Silurian	Cayugan		Chrysler Dol. Cobleskill Ls.	15 10
				Bertie	Williamsville Dol. Scajaquada Dol. Fiddlers Green Dol.	50
				Salina	Camillus Sh. Syracuse Sh. Vernon Sh.	200 200 600
Middle Silurian		Niagaran	Lockport Clinton	Sconondoa Ls. (Many formations)	80 to 150 320	
			Lower Silurian	Albion	Medina	Grimsby Ss.
Upper Ordovician				Queenston Sh.	700	

TABLE II

TABLE III
DETAILED DEVONIAN STRATIGRAPHIC COLUMN
SYRACUSE MERIDIAN - ONONDAGA DAM VICINITY

SYSTEM	SERIES	GROUP	FORMATION	MEMBER	APPROXIMATE THICKNESS IN FEET
Upper Devonian	Senecan		Tully Ls.		30
Middle Devonian	Erian	Hamilton	Moscow	Windom Sh.	165
				Portland Point Ls.	12
			Ludlowville	Owasco Ss.	2
				Spafford Sh.	27
				Ivy Point Ss.	37
				Otisco Sh.	209
				Centerfield Ls. & Ss.	25
			Skaneateles	Butternut Sh.	135
				Pompey Sh. & Siltst.	60
				Delphi Station Sh. & Siltst.	125
				Mottville Ls. & Sh.	6
			Marcellus	Cardiff Sh.	180
				Chittenango Bl. Sh.	80
				Cherry Valley Ls.	3
				Union Springs Bl. Sh.	12
			Onondaga Limestone	Seneca Ls.	25
				Tioga Bentonite	.4
				Moorehouse Ls*	25
				Nedrow Ls*	10
				Edgecliff Ls*	15
Lower Devonian	Ulsterian	Tristates	Oriskany Ss*		0 to .2
		Helderberg	Manlius	Jamesville Ls*	25
				Clark Reserv. Ls.	5
				Elmwood Dol.	10
				Olney Ls.	30
			Rondout	Chrysler Dol.	30

* = Exposed in spillway of dam.

TABLE III

TABLE IV
EARTHQUAKES WITHIN 200 KM OF ONONDAGA DAM SITE

Year	Date	Location		Maximum* Intensity	Richter Magnitude	Location
		Lat (N)	Long (W)			
1929	Aug. 12	42.9	78.4	VIII	5.8	Attica, NY
1954	Feb. 21	41.2	75.9	VII		Wilkes-Barre, PA
1840	Jan. 16	43.0	75.0	VI		Herkimer, NY
1853	Mar. 12	43.7	75.5	VI		Lowville, NY
1857	Oct. 23	43.2	78.6	VI		Buffalo, NY
1954	Feb. 24	41.2	75.9	VI		Wilkes-Barre, PA
1966	Jan. 1	42.8	78.2	VI	4.7	Attica, NY
1966	Jun. 13	42.9	78.2	VI	3.9	Attica, NY
1916	Feb. 2	42.9	74.0	V		Mohawk Valley, NY
1916	Feb. 3	43.0	74.0	V		Mohawk Valley, NY
1922	Dec. 8	44.4	75.1	V		S. of Canton, NY
1929	Dec. 2	42.8	78.3	V		Attica, NY
1952	Aug. 25	43.0	74.5	V		Johnstown, NY
1955	Jan. 21	43.0	73.8	V		Malta, NY
1955	Aug. 16	42.9	78.3	V		Attica, NY
1971	May 23	43.8	74.5	V	3.9	Blue Mt. Lake, NY
1971	May 23	43.9	74.5	V	3.6	Blue Mt. Lake, NY
1971	Jul. 10	43.9	74.5	V	3.4	Blue Mt. Lake, NY
1907	Jan. 24	42.8	74.0	IV		Schenectady, NY
1927	Mar. 14	44.6	75.4	IV		Canton, NY
1929	Dec. 3	42.8	78.3	IV		Attica, NY
1933	Oct. 29	43.0	74.7	IV		St. Johnsville, NY
1946	Oct. 28	41.5	76.6	IV	3.6	LaPorte, PA
1946	Nov. 10	42.9	77.5	IV	3.1	Canandaigua, NY
1950	Mar. 20	41.5	75.8	IV	3.3	NW of Scranton, PA
1954	Jan. 31	42.9	77.3	IV		Canandaigua, NY
1954	Feb. 1	43.0	76.7	IV	3.3	Montezuma, NY
1958	May 6	42.7	73.8	IV		Albany, NY
1960	Jan. 22	41.5	75.5	IV	3.4	Carbondale, PA
1963	Mar. 2	41.5	75.7	IV	3.4	Scranton, PA
1963	Jul. 2	42.6	73.8	IV	3.3	Albany, NY
1965	Jul. 16	43.0	78.1	IV	3.2	Attica, NY
1965	Aug. 27	43.0	78.1	IV	3.1	Attica, NY
1969	Aug. 13	43.3	78.2	IV	2.8	Albion, NY
1971	Jun. 21	43.9	74.5	IV	3.3	Blue Mt. Lake, NY
1973	Jul. 15	43.9	74.4	IV	3.4	Blue Mt. Lake, NY
1973	Jul. 15	43.9	74.4	IV	3.2	Blue Mt. Lake, NY
1973	Jul. 16	43.8	74.5	IV	3.3	Blue Mt. Lake, NY
1975	Nov. 3	43.9	74.6	IV	3.9	Raquette Lake, NY

* Modified Mercalli Intensity

Ref.: Chiburis (1981)

TABLE V

CHRONOLOGICAL LIST OF SIGNIFICANT ($I_o \geq VI$) EARTHQUAKES WITHIN NEW YORK

Year	Date	Location		Region (1)	I_o (2)	M (3)
		Lat (N)	Long (W)			
1737	Dec. 18	40.8	74.0	A	VII	
1775	July 6	43.5	73.5	B	VI	
1853	Mar. 12	43.7	75.5	B	VI	
1857	Oct. 23	43.2	78.6	C	VI	
1867	Dec. 18	44.7	75.2	B	VI	
1877	Nov. 4	45.3	74.0	B	VII	
1884	Aug. 10	40.6	74.0	A	VII	
1897	May 27	44.5	73.5	B	VI	
1929	Aug. 12	42.9	78.3	C	VIII	5.9
1931	Apr. 20	43.4	73.7	B	VII	5.0
1944	Sept. 5	45.0	74.9	B	VIII	5.9
1944	Sept. 5	45.0	74.9	B	-	4.6
1957	Mar. 23	41.6	74.8	A	VI	4.8
1966	Jan. 1	42.8	78.2	C	VI	4.7
1967	Jun. 13	42.9	78.2	C	VI	3.9
1974	Jun. 7	41.6	73.9	A	-	3.3
1975	Jun. 9	44.9	73.6	B	-	4.2

1. See Figure 6.

2. I_o = Maximum Intensity (Modified Mercalli Scale)

3. M = Generalized Magnitude

Ref.: Mitronovas (1981)

TABLE VI

LIST OF EARTHQUAKES HAVING ATTENUATED
INTENSITIES GREATER THAN OR EQUAL TO IV
AT THE ONONDAGA DAM SITE

EVENT	REGION	AT EPICENTER		DISTANCE FROM SITE (KM)	ATTENUATED INTENSITY ⁽²⁾
		Intensity ⁽¹⁾	Magnitude		
Feb. 5, 1663	La Malbaie, Quebec	10		705	4.7
Aug. 12, 1929	Attica, N. Y.	8	5.8	182	4.4
Sept. 5, 1944	Massena, N. Y.	8	5.9	252	4.0
Oct. 20, 1870	Baie-St.-Paul, Quebec	9		668	3.8
Mar. 1, 1925	La Malbaie, Quebec	9	6.6	705	3.7
June 11, 1638	La Malbaie, Quebec	9		707	3.7
1534	Les Eboulements, Quebec	9		713	3.7
Nov. 18, 1929	Grand Banks, NF	10	7.2	1608	3.6
Sept. 16, 1732	Montreal, Quebec	8		352	3.6

(1) Modified Mercalli Intensity

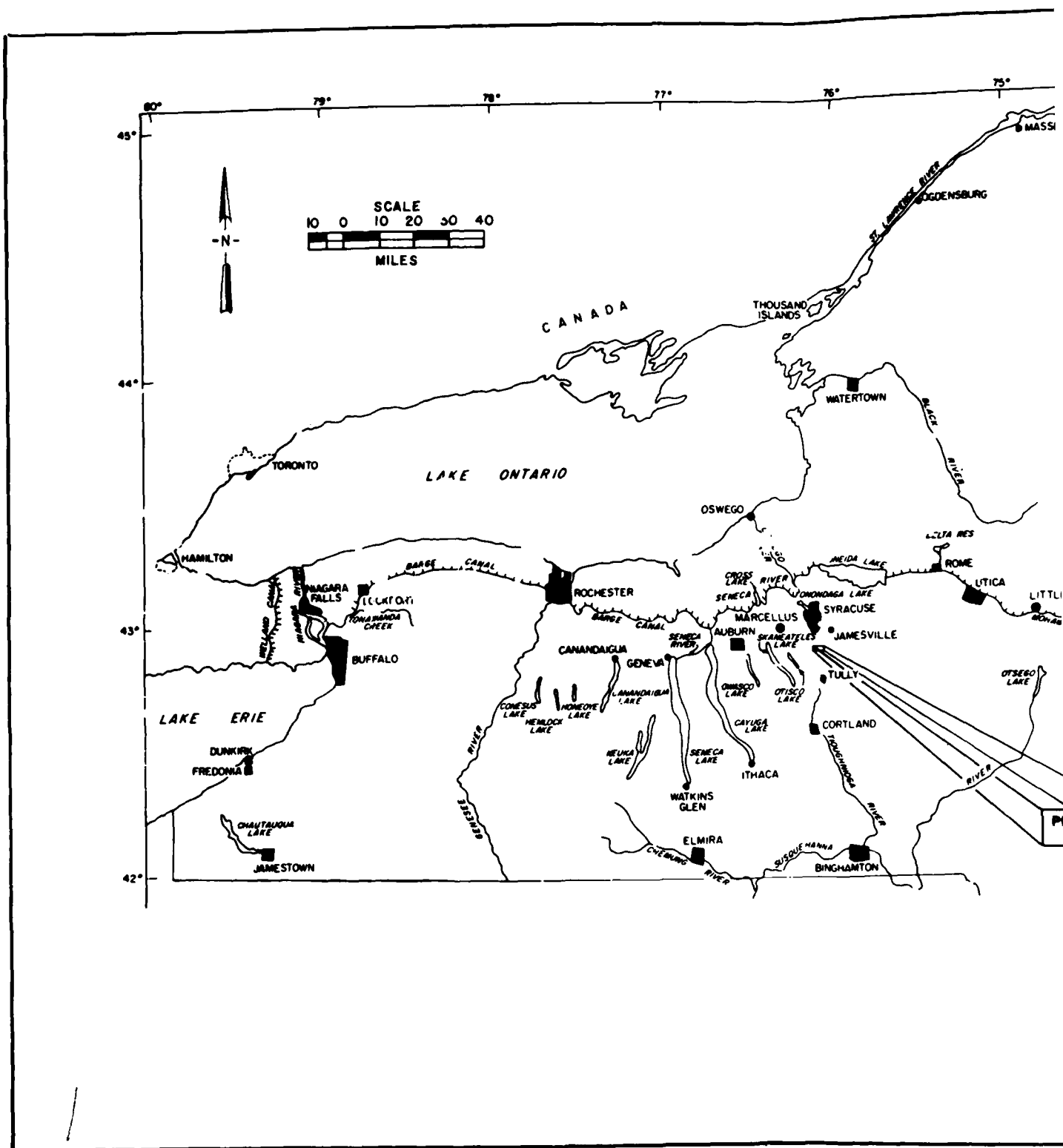
(2) At Dam Site

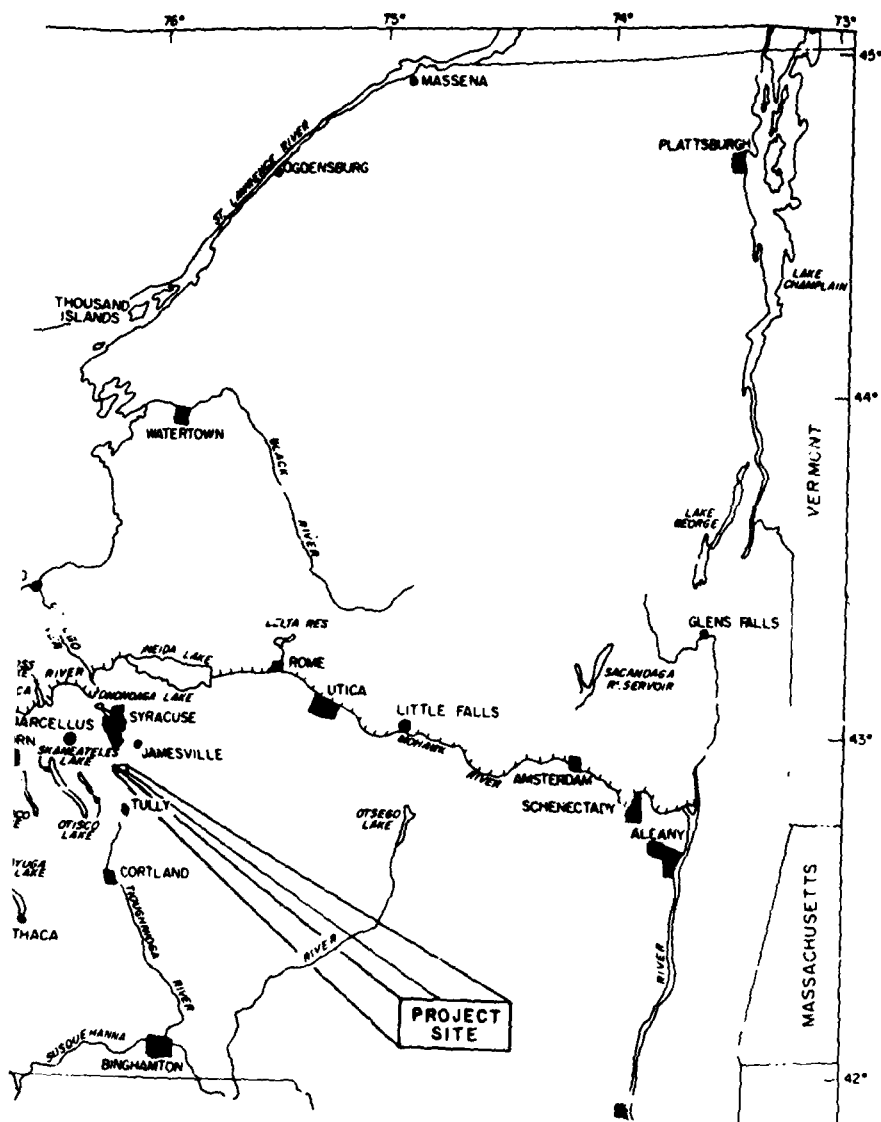
Ref: Chiburis (1981)

ONONDAGA DAM, NEW YORK

FIGURES

U. S. Army Engineer District, Buffalo
1776 Niagara Street
Buffalo, New York 14207



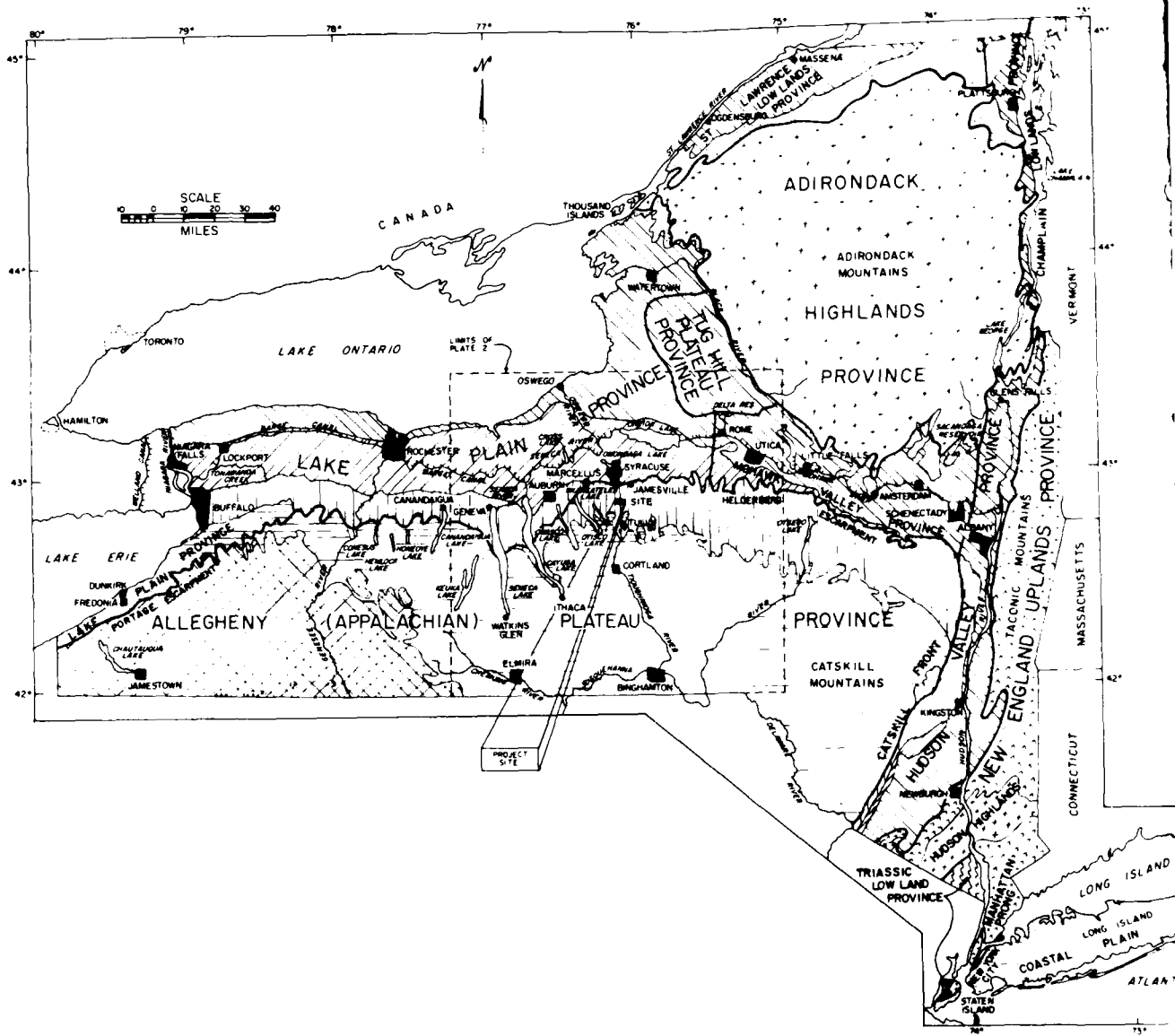


ONONDAGA DAM, NEW YORK

PROJECT LOCUS

U.S. ARMY ENGINEER DISTRICT BUFFALO
 TO ACCOMPANY PHASE I, SEISMIC STABILITY INVESTIGATION
 DATED: AUGUST 1982

FIGURE 1



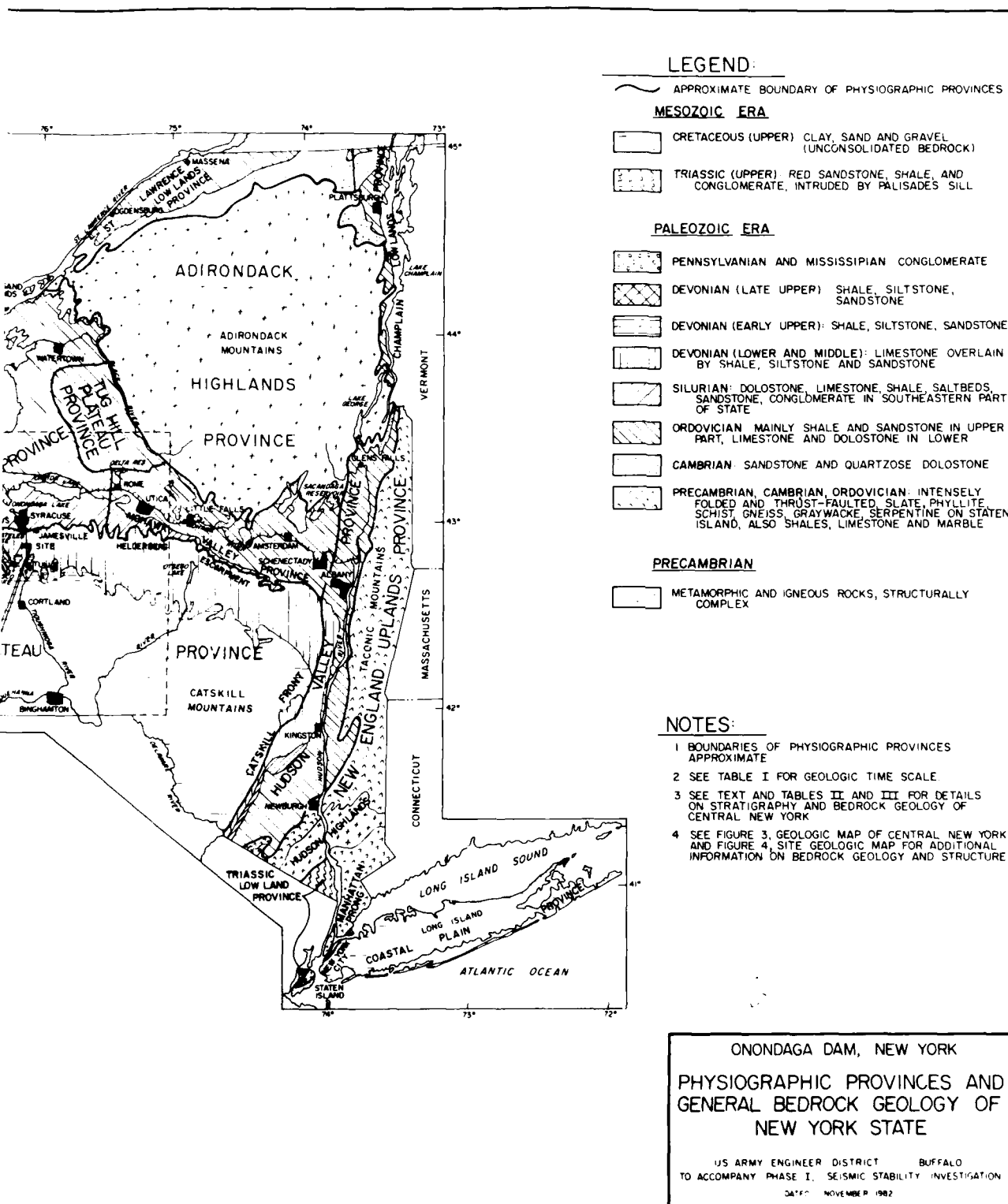
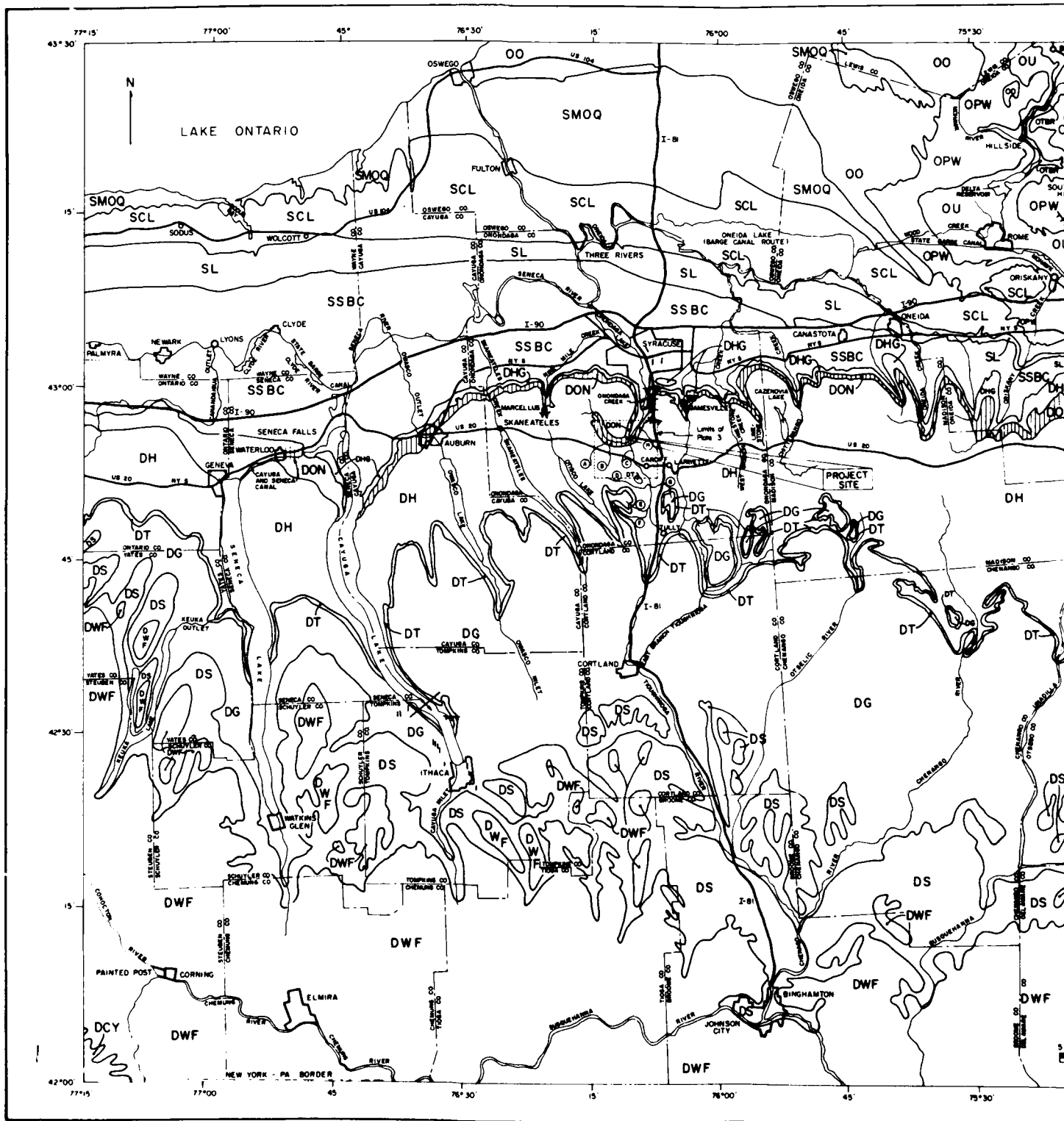


FIGURE 2



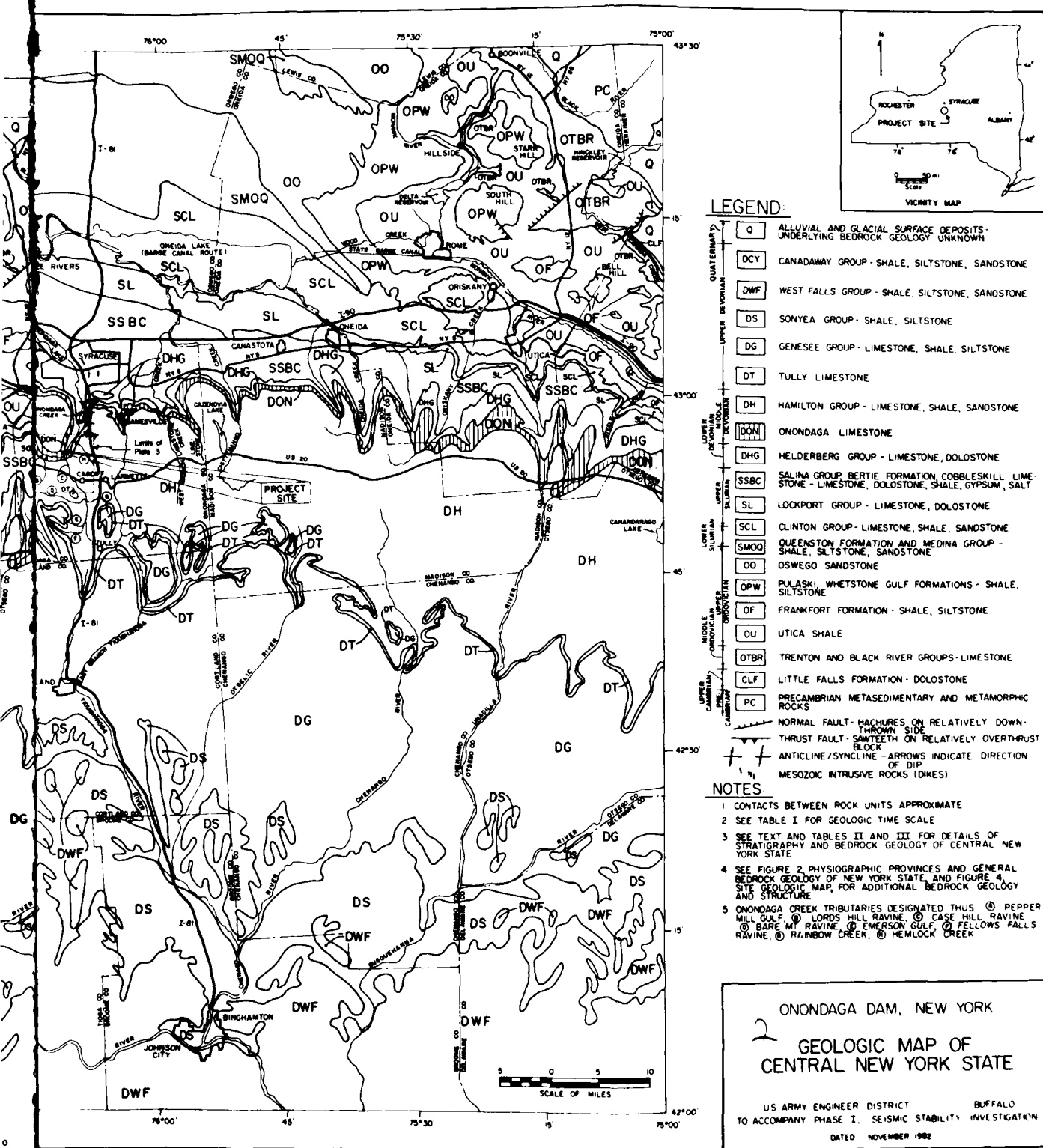
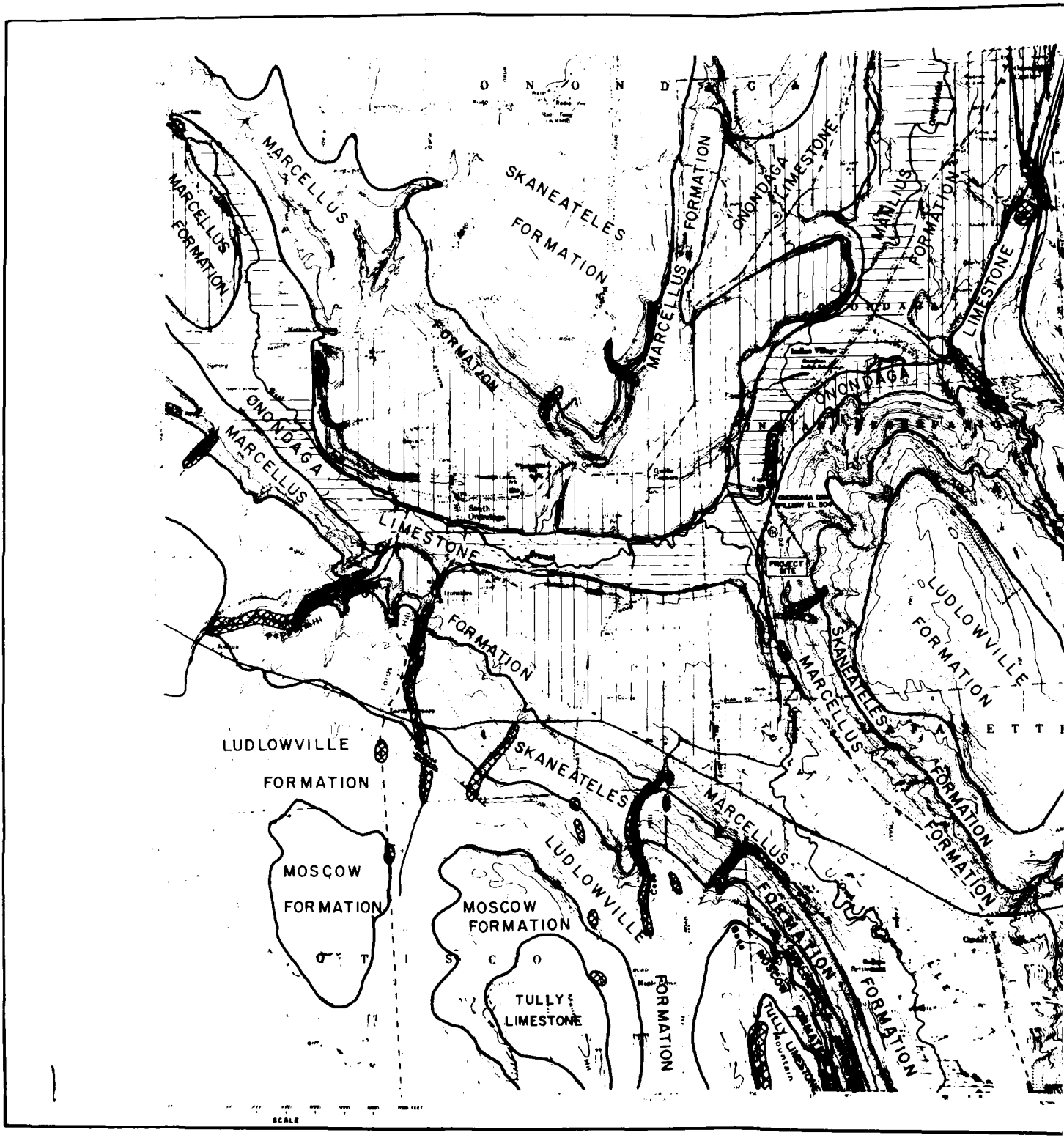


FIGURE 3



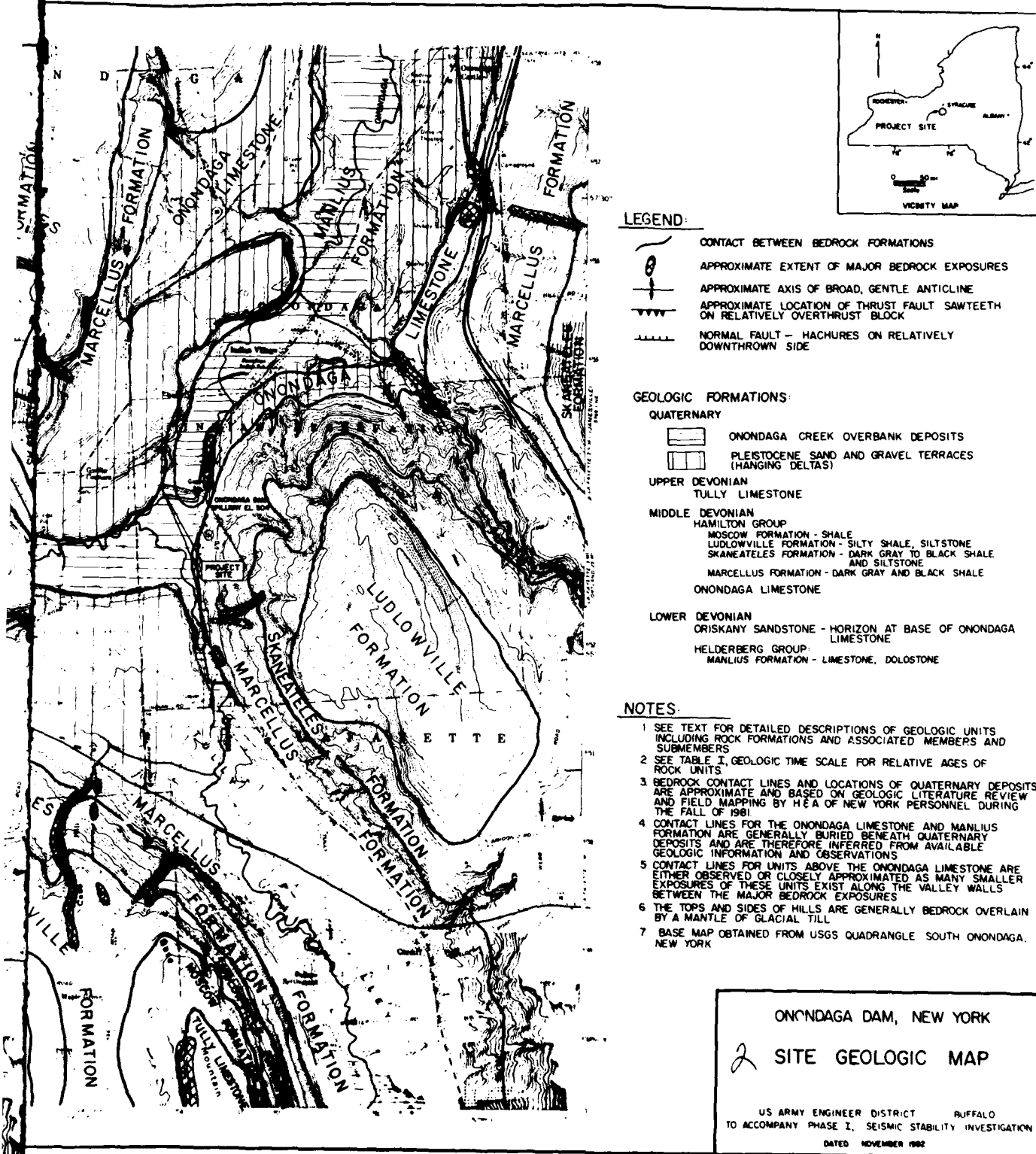
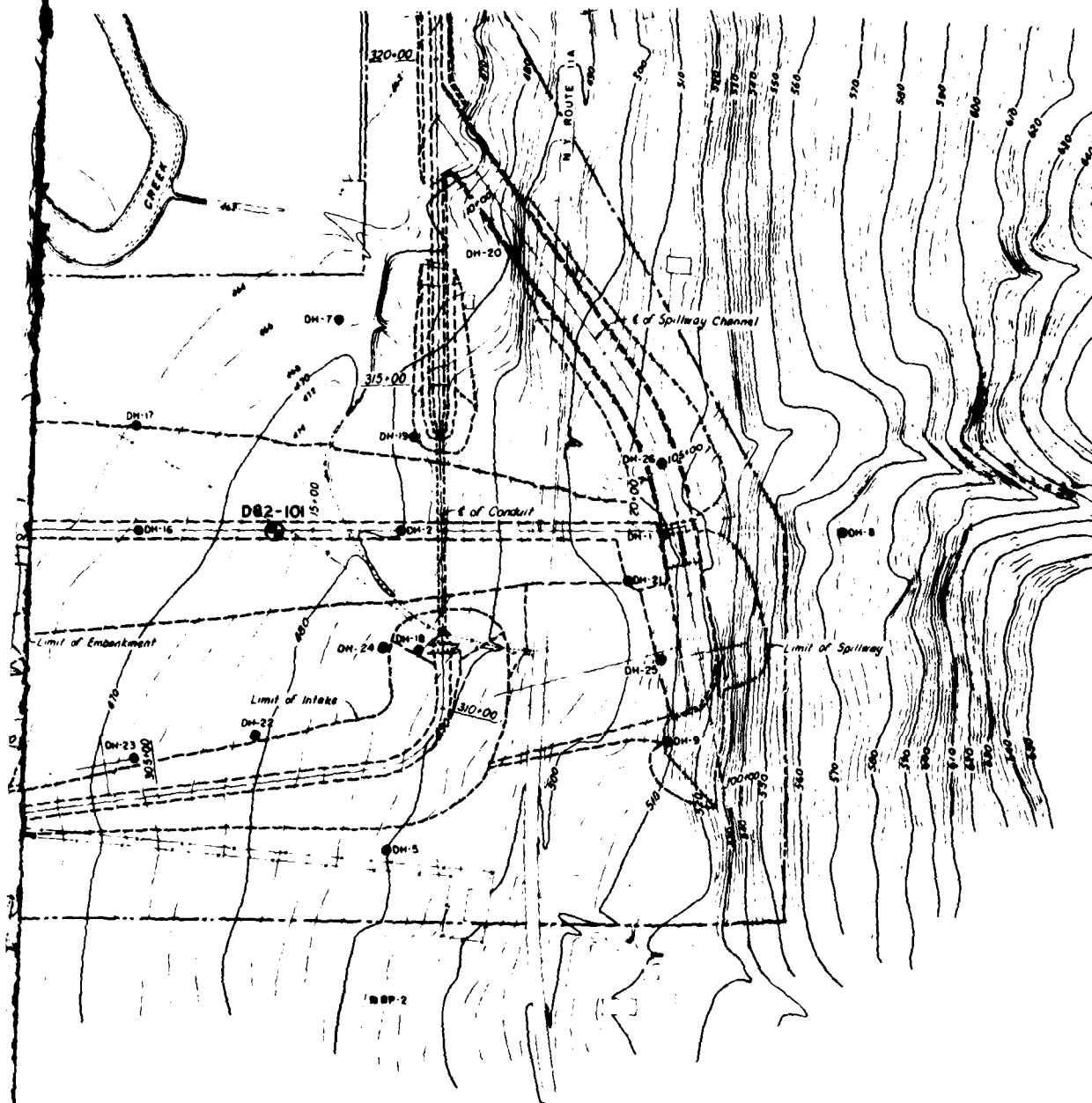


FIGURE 4

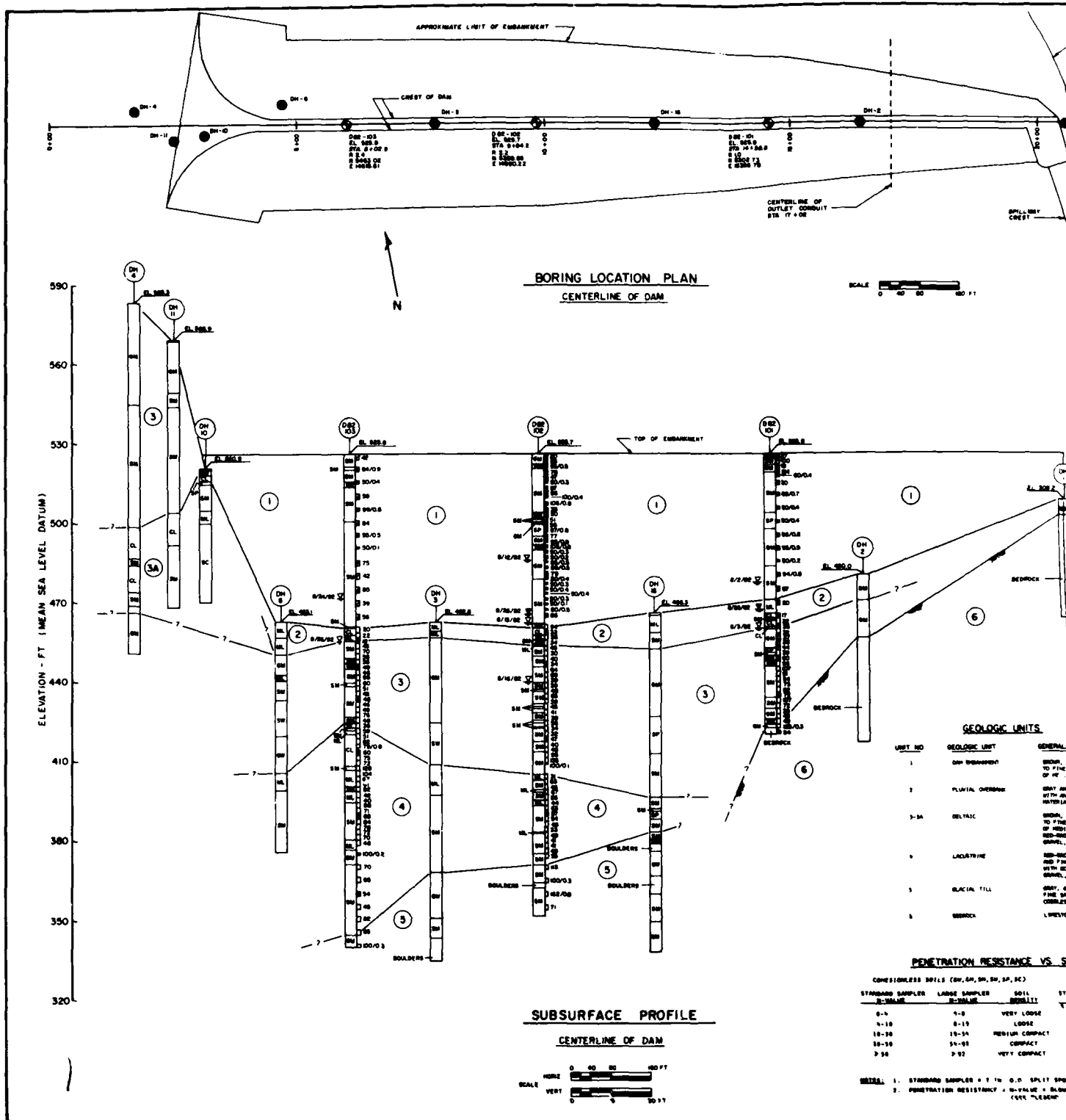


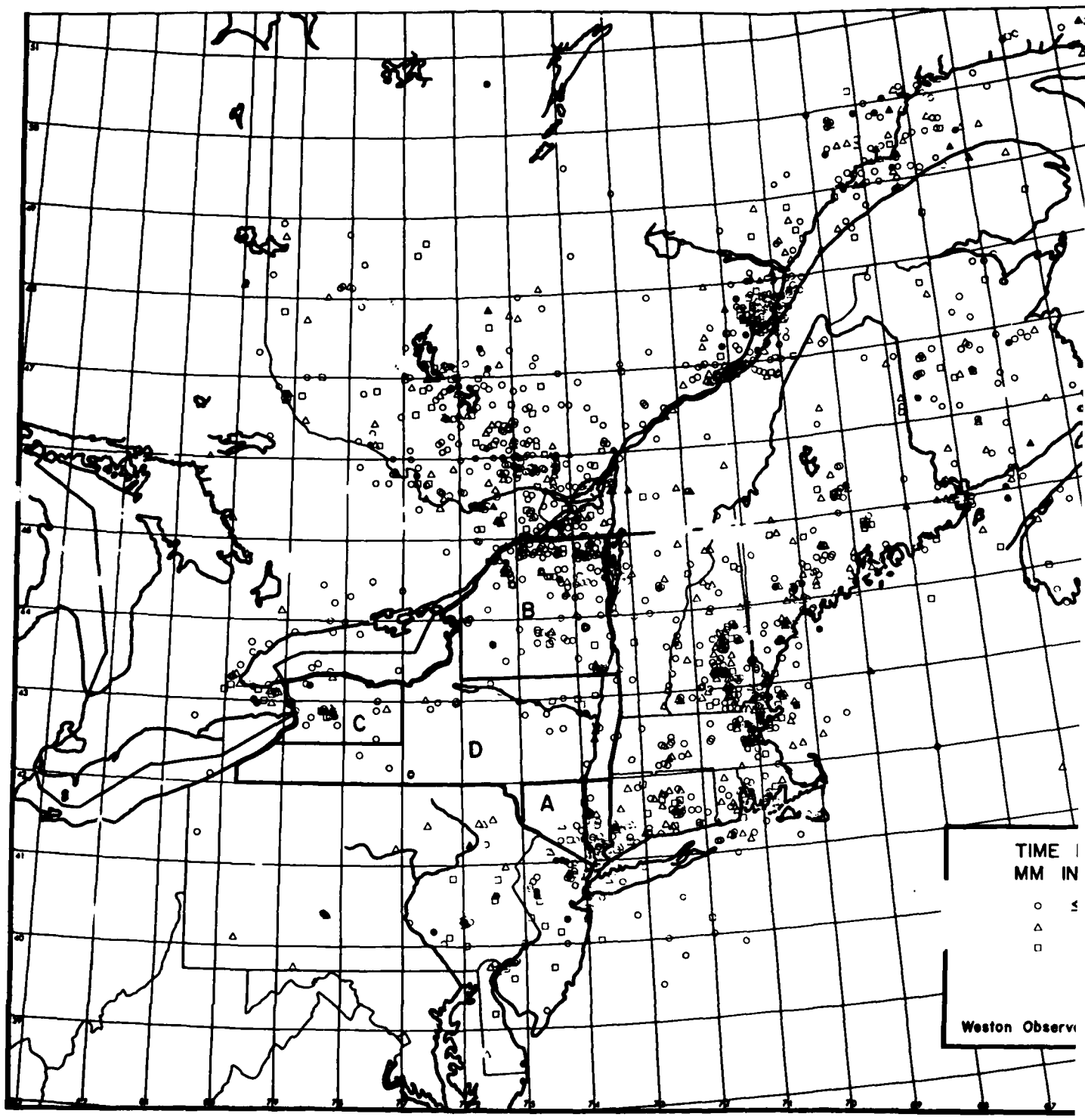
I. REF. U.S. ARMY CORPS OF ENGINEERS.
(1947)

ONONDAGE DAM, NEW YORK SITE AND SUBSURFACE EXPLORATION PLAN

U.S. ARMY ENGINEER DISTRICT BUFFALO
TO ACCOMPANY PHASE I, GEOTECHNICAL INVESTIGATION
DATED AUGUST 1958

FIGURE 5

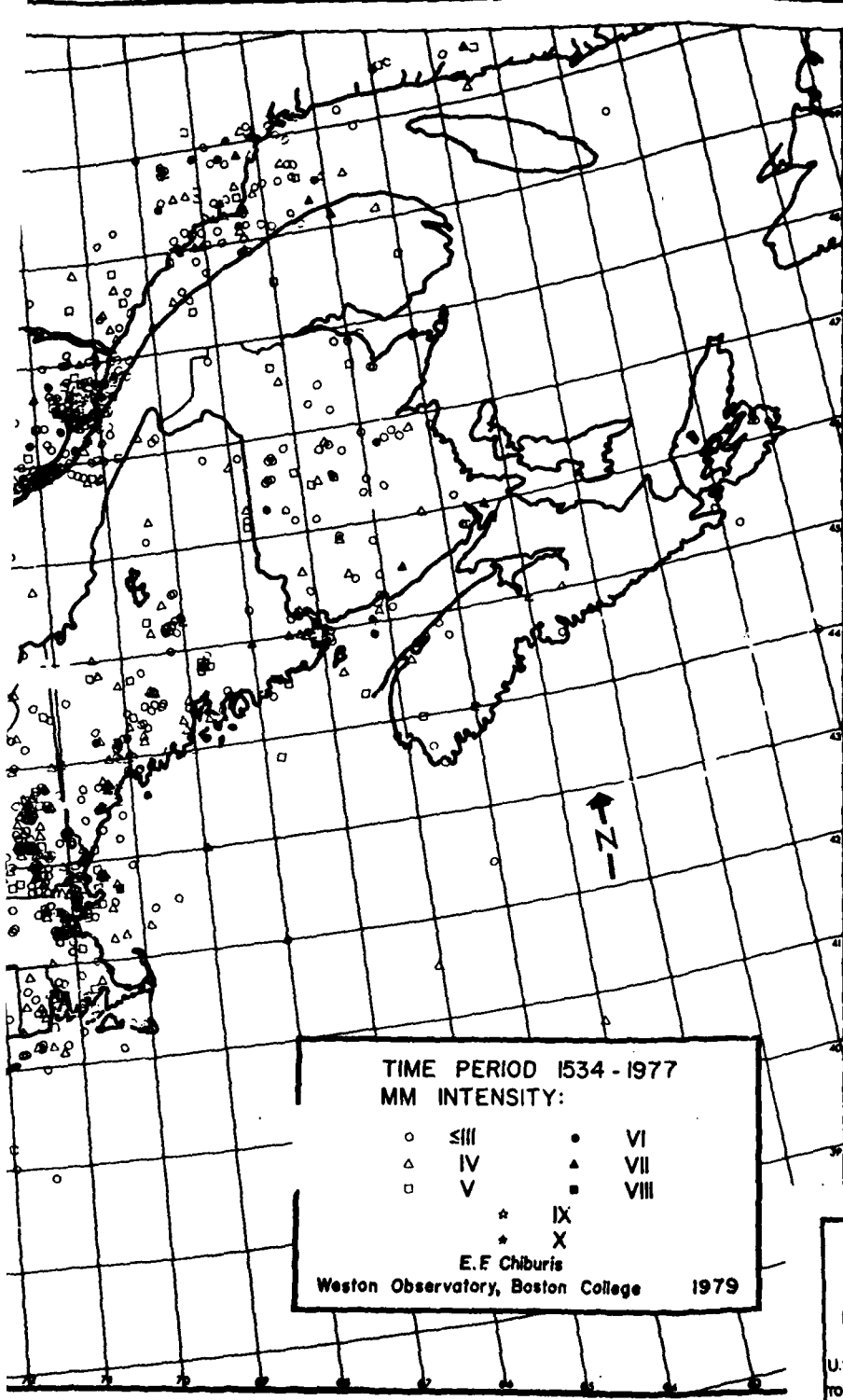




TIME
MM IN

O
Δ
□

Western Observ



NOTE: ZONES A, B, C AND D
OBTAINED FROM
MITRONOVAS (1981)

REF: Chiburis, et al (1979)

2

ONONDAGA DAM, NEW YORK
SEISMICITY EPICENTERS
NORTHEASTERN U.S. VICINITY

U.S. ARMY ENGINEER DISTRICT BUFFALO
TO ACCOMPANY PHASE I, SEISMIC STABILITY INVESTIGATION
DATED: AUGUST 1982

FIGURE 7

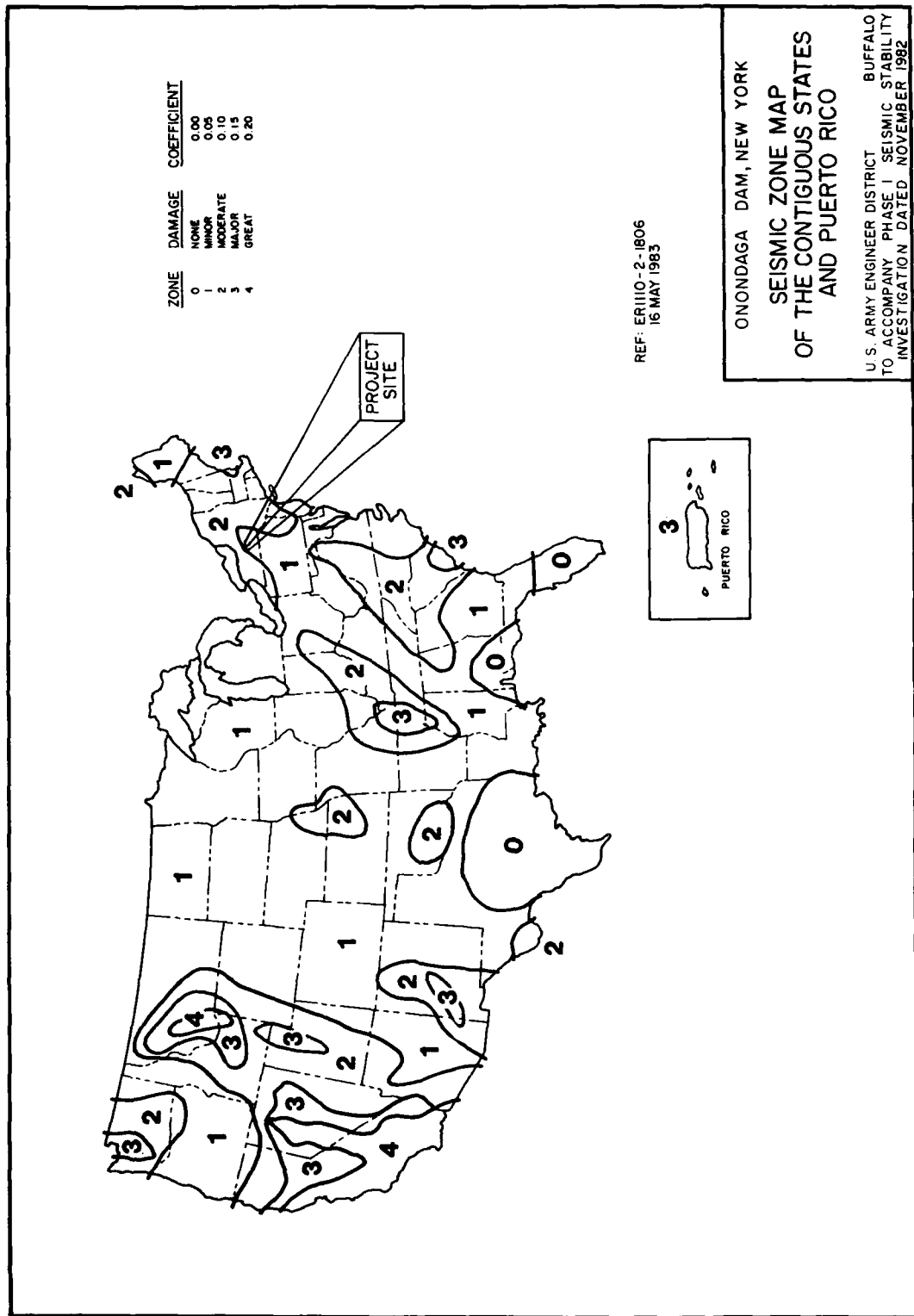


FIGURE 7A

FIGURE 7A

MAGNITUDE <i>M</i>	EPICENTRAL ACCELERATION <i>a_e</i>	MODIFIED-MERCALLI INTENSITY SCALE <i>I</i>	GROUND ACCELERATION <i>a</i>
	$\frac{\text{cm}}{\text{sec}}$ $\frac{a_0}{g}$	<i>I</i> Detected only by sensitive instruments.	$\frac{\text{cm}}{\text{sec}}$ $\frac{a}{g}$
		<i>II</i> Felt by a few persons at rest, especially on upper floors; delicate suspended objects may swing.	-2 -3
<i>M</i> =3	3 4 5 .005g	<i>III</i> Felt noticeably indoors, but not always recognized as a quake; standing autos rock slightly, vibration like passing truck.	4 5 .005g 6 7
		<i>IV</i> Felt indoors by many, outdoors by a few; at night some awakened; dishes, windows, doors disturbed; motor cars rock noticeably.	8 9 10 .01g
<i>M</i> =4	10 .01g	<i>V</i> Felt by most people; some breakage of dishes, windows, and plaster; disturbance of tall objects.	20 30
		<i>VI</i> Felt by all; many frightened and run outdoors; falling plaster and chimneys; damage small.	40 50 .05g 60 70
<i>M</i> =5	60 .05g 70 80 90 0.1g 100	<i>VII</i> Everybody runs outdoors; damage to buildings varies, depending on quality of construction; noticed by drivers of autos.	80 90 100 0.1g
<i>M</i> =6	200	<i>VIII</i> Panel walls thrown out of frames; fall of walls, monuments, chimneys; sand and mud ejected; drivers of autos disturbed.	200 300
		<i>IX</i> Buildings shifted off foundations, cracked, thrown out of plumb; ground cracked; underground pipes broken.	400 500 .5g 600
<i>M</i> =7	500 .5g 600 700 800 900 1000 1g	<i>X</i> Most masonry and frame structures destroyed; ground cracked; rails bent; landslides.	700 800 900 1000 1g
		<i>XI</i> New structures remain standing; bridges destroyed; fissures in ground; pipes broken; landslides; rails bent.	2000 3000
<i>M</i> =8	2000 3000 4000 .5g	<i>XII</i> Damage total; waves seen on ground surface; lines of sight and level distorted; objects thrown up into air.	4000 5000 .5g 6000

REF: Nuclear Reactors and Earthquakes (1963)

ONONDAGA DAM, NEW YORK INTENSITY, MAGNITUDE AND ACCELERATION RELATIONSHIP

U.S. ARMY ENGINEER DISTRICT BUFFALO
TO ACCOMPANY PHASE I, SEISMIC STABILITY INVESTIGATION
DATED: AUGUST 1962

FIGURE 8

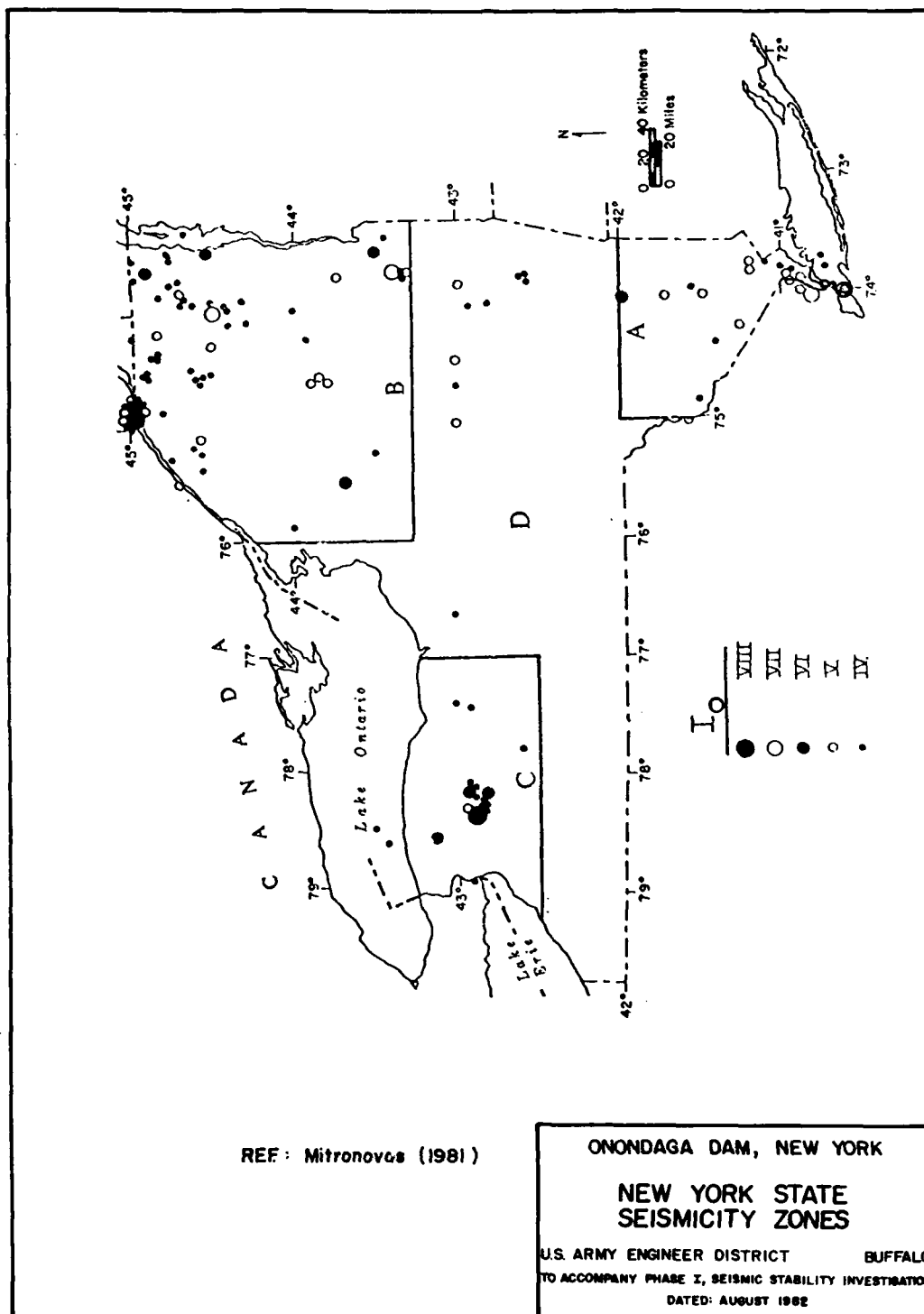


FIGURE 9

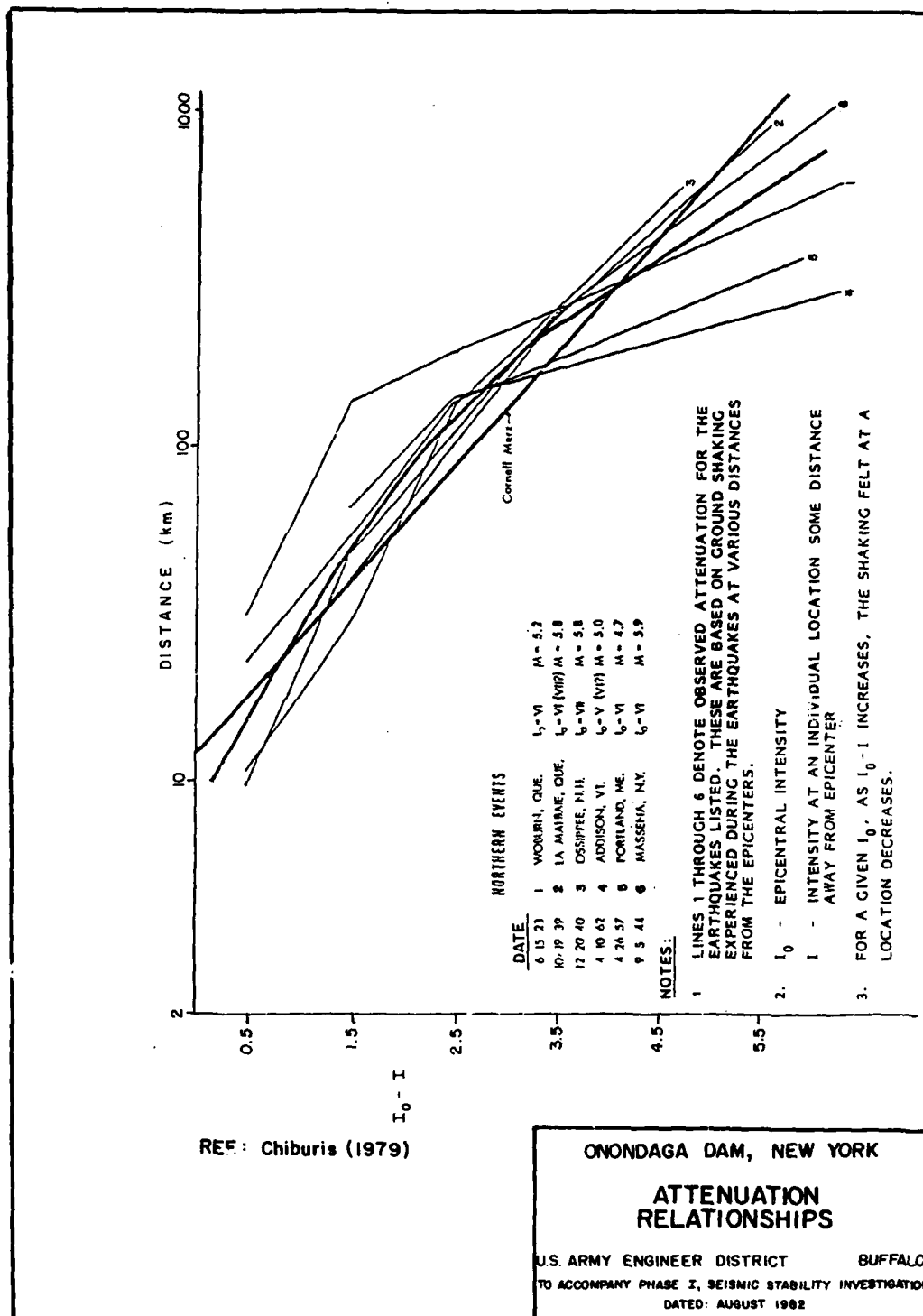
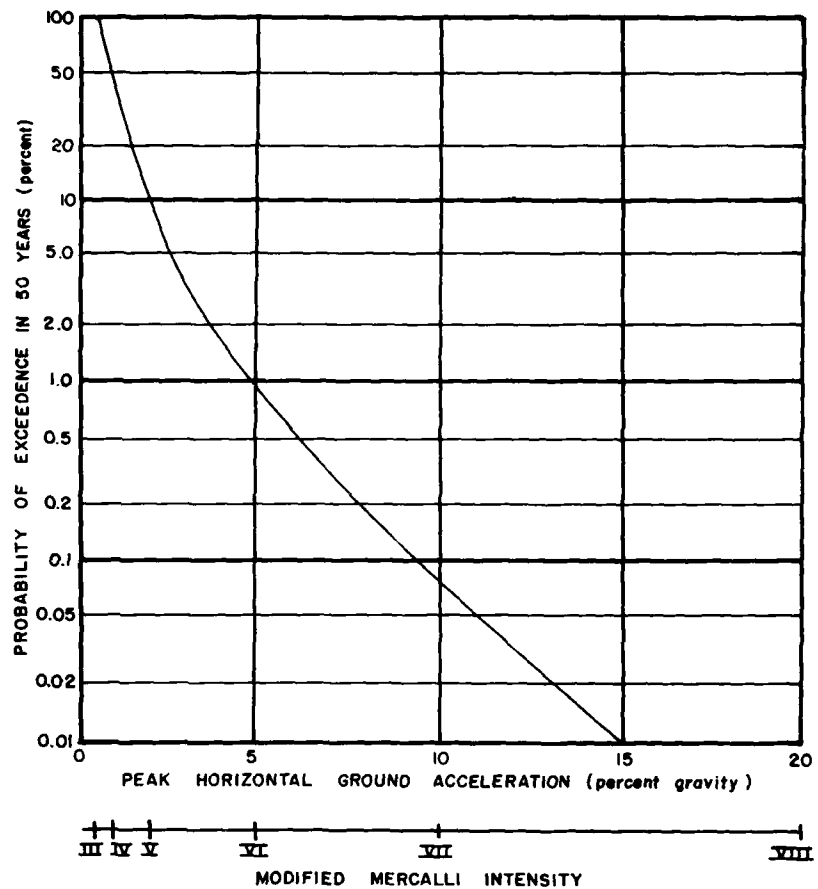


FIGURE II



NOTE:

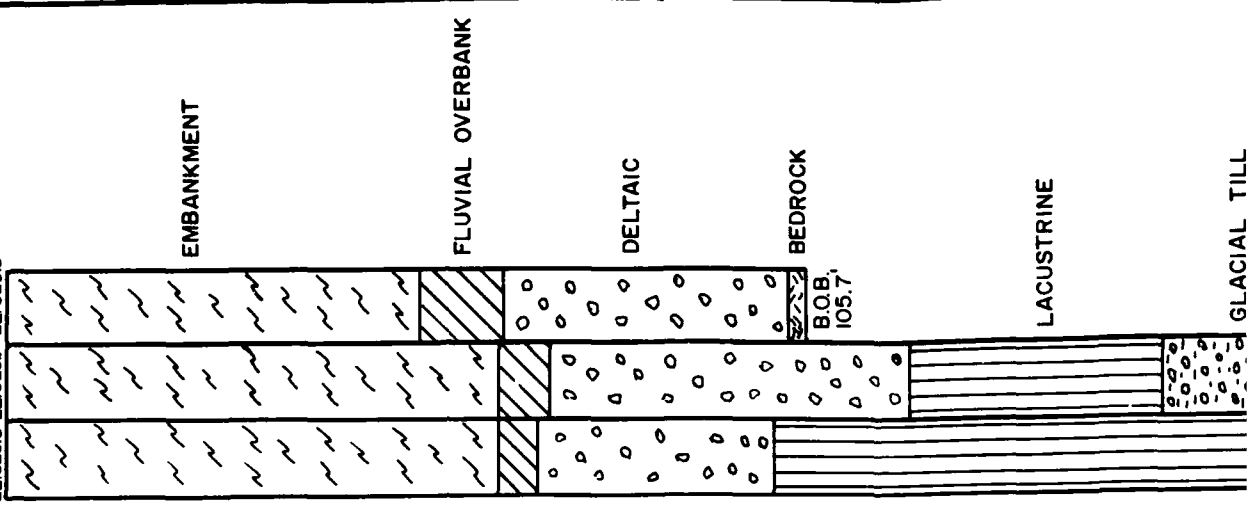
BASED ON CORNELL AND MERZ (1975)
ATTENUATION ANALYSES USING
STANDARD DEVIATION OF 0.4

ONONDAGA DAM, NEW YORK
PROBABILITY OF EXCEEDENCE
FOR PEAK HORIZONTAL
GROUND ACCELERATIONS
U.S. ARMY ENGINEER DISTRICT BUFFALO
TO ACCOMPANY PHASE I, SEISMIC STABILITY INVESTIGATION
DATED: AUGUST 1982

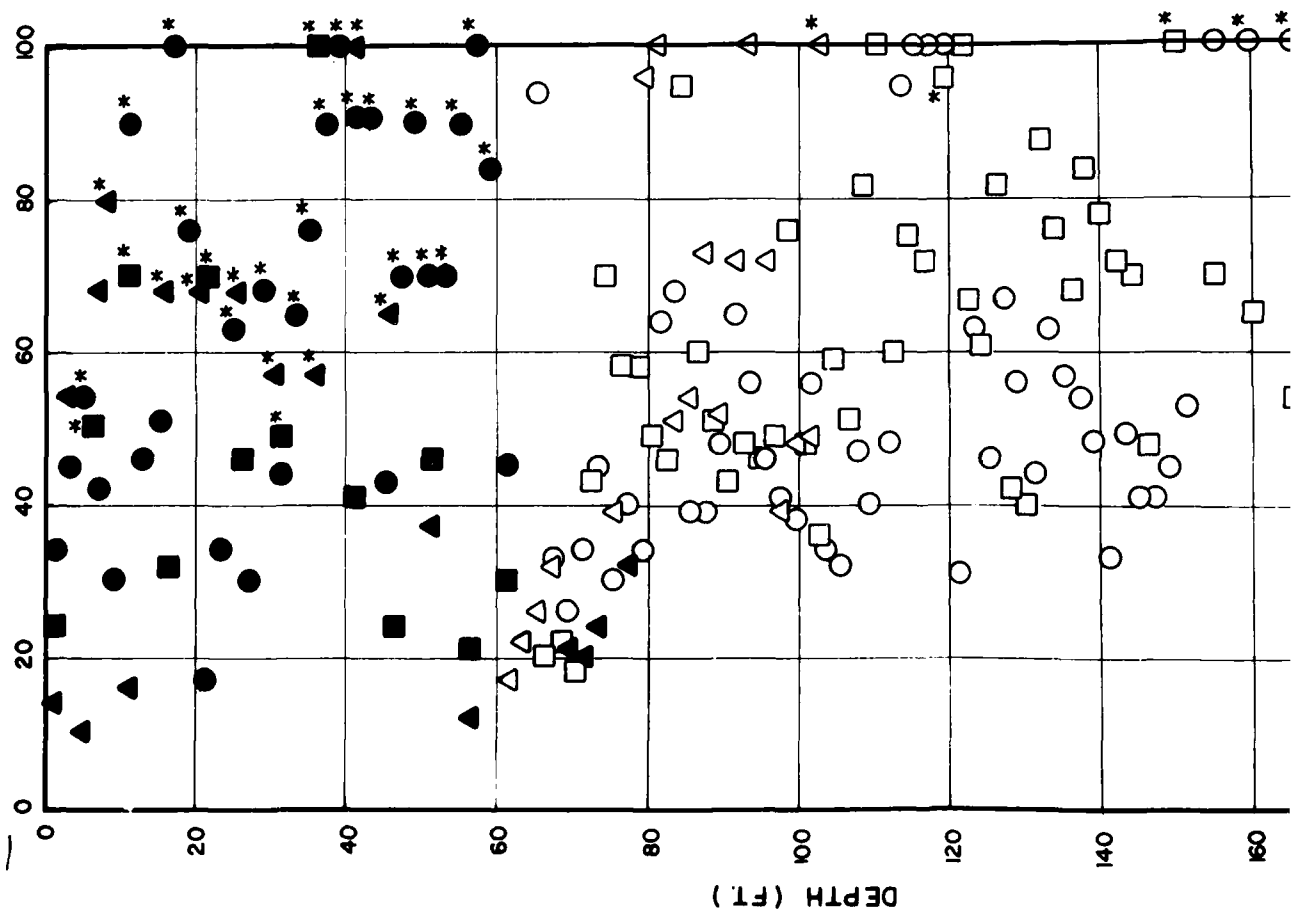
FIGURE 12

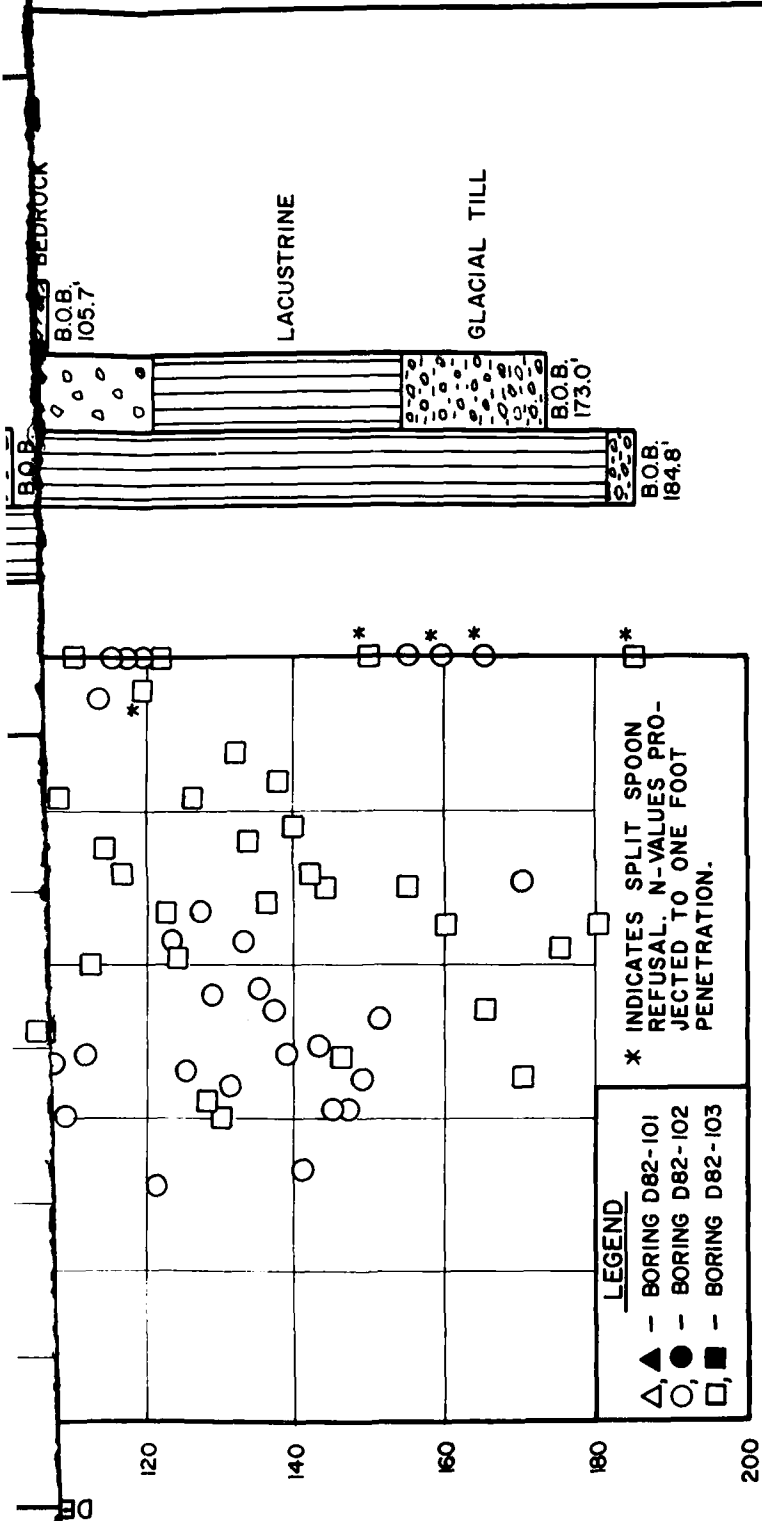
**SCHEMATIC
BORING LOGS**

D82 D82 D82
103 102 101
EL. 526.8 EL. 525.7 EL. 525.8



**STANDARD PENETRATION RESISTANCE
N - VALUE (BLOWS/FT.)**





NOTES:

1. OPEN SYMBOL INDICATES USE OF 1-3/8 IN. I.D. x 2.0 IN. O.D. STANDARD SPLIT SPOON SAMPLER DRIVEN BY 140-LB. HAMMER FALLING FREELY 30 INCHES.
2. CLOSED SYMBOL INDICATES USE OF 2-7/8 IN. I.D. x 3.5 IN. O.D. SPLIT SPOON SAMPLER DRIVEN BY 350-LB. HAMMER FALLING FREELY 18 INCHES. N-VALUES HAVE BEEN CONVERTED TO EQUIVALENT STANDARD N-VALUE (SEE APPENDIX F).
3. N-VALUES GREATER THAN 100 HAVE BEEN PLOTTED AT 100 ON THE CHART.
4. SEE TEXT FOR DESCRIPTION OF SOIL DEPOSITS.

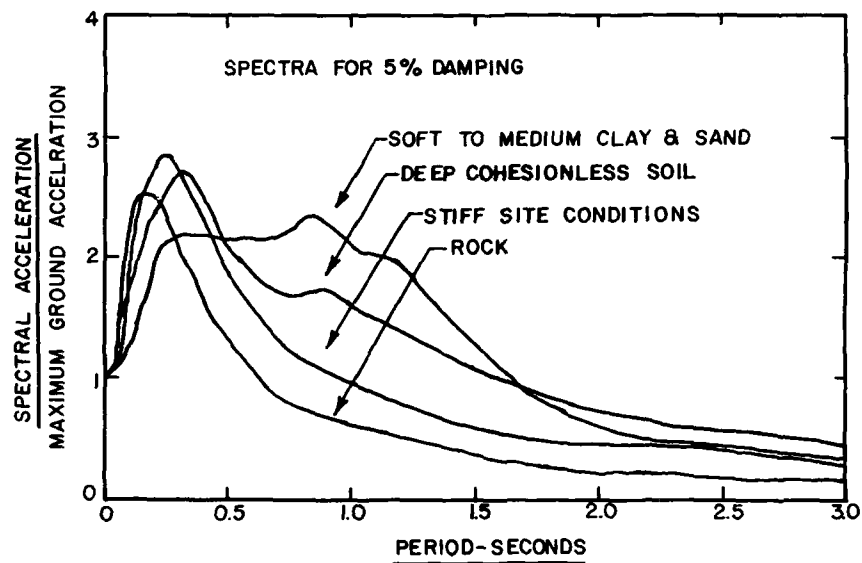
ONONDAGA DAM, NEW YORK

STANDARD PENETRATION RESISTANCE VS. DEPTH

U.S. ARMY ENGINEER DISTRICT BUFFALO
TO ACCOMPANY PHASE I, SEISMIC STABILITY INVESTIGATION
DATED: SEPTEMBER 1982

FIGURE 13

FIGURE 13



REF: APPLIED TECHNOLOGY COUNCIL
(PUBLICATION ATC 3-06, 1978)

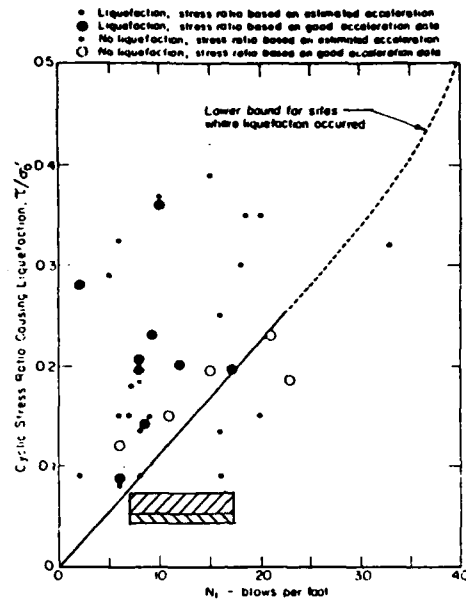
ONONDAGA DAM, NEW YORK

AVERAGE ACCELERATION
SPECTRA FOR DIFFERENT
SITE CONDITIONS

U.S. ARMY ENGINEER DISTRICT, BUFFALO
TO ACCOMPANY PHASE I, SEISMIC STABILITY
INVESTIGATION. DATED: NOVEMBER 1982

FIGURE 14

FILE NO. 7450 03



LEGEND:



SOILS BELOW TOE OF DAM



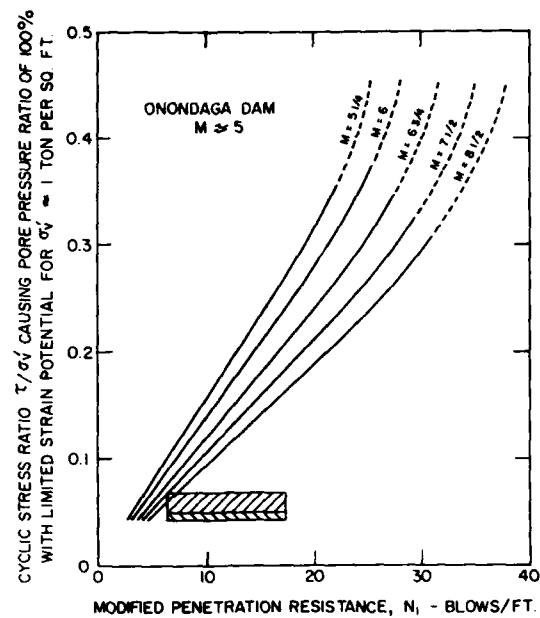
SOILS BELOW CREST OF DAM

REF: SEED AND IDRISS (1981)

ONONDAGA DAM, NEW YORK
 LIQUEFACTION POTENTIAL OF
 DAM FOUNDATION SOILS

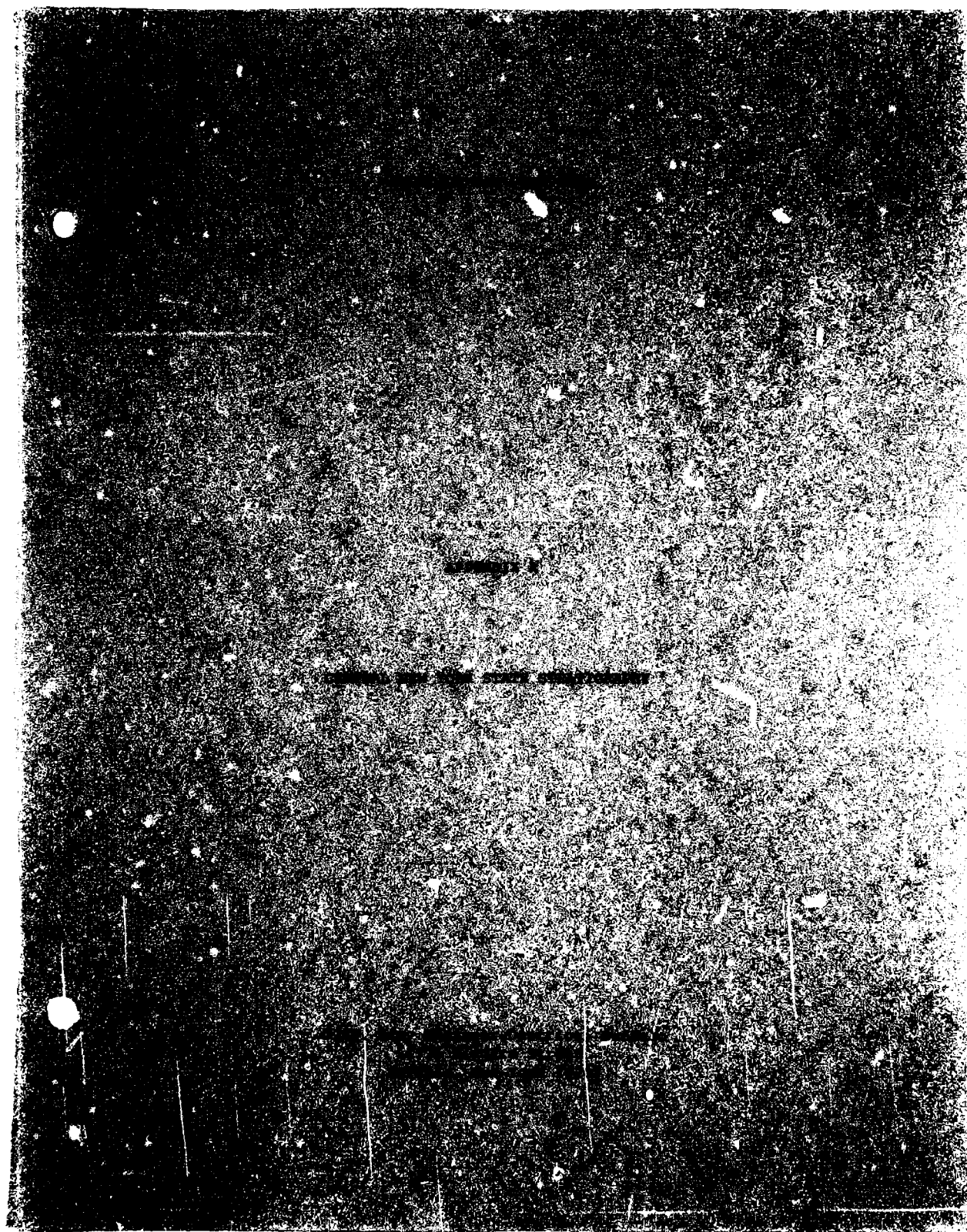
U.S. ARMY ENGINEER DISTRICT BUFFALO
 TO ACCOMPANY PHASE I, SEISMIC STABILITY INVESTIGATION
 DATED: AUGUST 1982

FIGURE 15



ONONDAGA DAM, NEW YORK
 LIQUEFACTION POTENTIAL
 OF DAM FOUNDATION SOILS
 CORRELATED TO DIFFERENT
 EARTHQUAKE MAGNITUDES
 U. S. ARMY ENGINEER DISTRICT BUFFALO
 TO ACCOMPANY PHASE I, SEISMIC STABILITY
 INVESTIGATION DATED: NOVEMBER 1982

FIGURE 15A



APPENDIX A

CENTRAL NEW YORK STRATIGRAPHY

A1. Table II presents the generalized stratigraphic column for central New York and Figure 3 is a geologic map of the central New York Region.

A2. As shown on Table II, the rocks in central New York range in age from Late Ordovician along the south shore of Lake Ontario to Late Devonian at the New York-Pennsylvania border. Refer to Table I for the geologic time scale.

Upper Ordovician System

A3. The oldest stratigraphic unit in central New York, exposed along the south shore of Lake Ontario, is the Queenston Formation. It encompasses a maximum of 700 feet of red, crumbly shale and red siltstone and sandstone in this region. The coarser beds increase in thickness eastward and southward.

Silurian System

A4. Silurian rocks surround the Appalachian Plateau on the north and are found in the Lake Ontario Plain, and on the east in the westernmost Hudson Valley Province. In the Helderberg region in the northeast corner of the Allegheny Plateau, Lower Devonian rocks rest upon the Middle Ordovician and the normally intervening Silurian strata are entirely missing.

A5. For convenience, the New York Silurian can be subdivided into three parts. A lower portion, mainly of red sandstone beds (Medina Group); a middle part consisting of shales and carbonates (Clinton and Lockport Groups); and an upper division of more unusual strata, mostly hypersaline evaporites and brackish lagoonal deposits (Salina and Bertie Groups).

A6. Complexities in the Silurian rocks include numerous facies changes from west to east, the presence of many depositional breaks (disconformities), and poor exposures especially in the Upper Silurian section. Small fossils called conodonts and ostracods have been used to develop refined correlations in the Silurian System of New York State, and individual strata can now be identified from rock core of exploratory borings. These rocks have a simple geologic structure, dipping gently southward at 40 to 50 feet per mile (65 to 80 feet in the subsurface) south of Lake Ontario.

Albion Series (Lower Silurian)

A7. Lower Silurian rocks are exposed in a narrow east-west belt parallel to the south shore of Lake Ontario and just south of the exposure belt of Queenston rocks, pinching out by unconformity east of the Oswego River. The line of Barge Canal eastward from Lockport to Spencerport follows the outcrops of this unit.

A8. The Medina Group is the sole Albion-aged unit in New York State. It is made up of, in the Niagara region, the Whirlpool Sandstone (25 feet thick) below, Power Glen Formation (30 feet thick) and the Grimsby Formation or Red Medina Sandstone above (50 to 70 feet thick). East of Lockport, the Whirlpool and Power Glen rocks pass by facies change into the lower Grimsby Formation. At Rochester, the Grimsby Formation rests directly on the Queenston Formation.

A9. The Grimsby Formation consists of red, crossbedded, medium bedded to massive, medium to coarse, locally conglomeritic, arkosic sandstone with some red silty shale. East of Holly and Medina, the massive sands become thin and interbedded with finer silty shales. To the west, the silty shales become thicker.

Niagaran Series (Middle Silurian)

A10. Niagaran rocks are exposed in an east-west belt from the Niagara River through Rochester to the Helderberg region and in a northeast-

southwest belt in the western Hudson Valley Province from the Kingston area to Port Jervis. Niagaran rocks are missing around the Northeast perimeter of the Allegheny Plateau. On the Lake Ontario Plain, the Niagaran Series embraces the Clinton Group below and the Lockport Group above.

A11. The Clinton Group is a complex suite of lithologies and facies changes, with numerous disconformities. Therefore, no Clinton formation traverses the state without some change in character. In general, coarse clastic sedimentary rocks dominate this section in east central New York, with fine shales and carbonates being restricted to western New York. The section is about 120 feet thick at Niagara, 180 feet in the Rochester Gorge, 320 feet in the Oneida Lake region, and some 220 feet at its easternmost outcrops in Oneida and Herkimer Counties. It can be divided into a lower, middle and upper part based on fossil zones.

A12. Lower Clinton rocks represent a transgressive sequence of rock types, including the basal Kodak (Thorold) Sandstone (six feet thick), Oneida conglomerate (12 to 50 feet), the Neahga (six feet), Maplewood (20 feet), Bear Creek (10 feet), and Sodus (60 feet) shales, and finally terminating upward with Reynales (7 to 17 feet) and Wolcott Limestone (20 feet).

A13. Middle Clinton formations are restricted to central and east central New York, none being found west of the Sodus Bay - Seneca Lake meridian. The Sauquoit Formation is the sole Middle Clinton unit (0 to 80 feet), consisting of shales with interbedded sandstone and siltstones. The Sauquoit strata change eastward into the Otsquago Sandstone (100 to 130 feet). At the top of the Sauquoit, the fourth Clinton iron ore bed is found, the Westmoreland, south of Rome and Utica. The Otsquago and the eastern and coarsest Sauquoit beds have yielded several gas and salt water shows in Madison and Otsego Counties.

A14. Upper Clinton rocks, unlike the Middle Clinton units, are found across New York, east to west. The Irondequoit Limestone (18 to 30 feet), Williamson Black Shale (0 to 30 feet) and Willowvale Shale

(0 to 80 feet) are all facies of one another comprising the lowermost portion. In eastern New York, the Kirkland Hematite rests on top of the Dawes Dolostone (ten feet), the uppermost Clinton iron ore. The most significant portion of the Upper Clinton is made up of the Rochester Shale and its eastern facies, the Herkimer Sandstone. Rochester rocks are relatively thick, gray to dark gray, calcareous, dolomitic, silty shale containing numerous limestone and dolostone beds and lenses, especially in the upper portion. In the Genesee region, an upper dolostone is so prominent that a separate mappable unit, the Gates Dolostone, is recognized. The Rochester Shale is about 90 feet thick in the west and increases to 140 feet thick in central New York, north of Syracuse.

A15. The Herkimer Sandstone is 70 feet of gray calcareous, thin-bedded sandstone, interbedded with dark gray sandy and silty shale, and is found only in the Oneida-Utica region. To the east, this sandstone becomes a coarse, medium-bedded, gray and red sandstone with minor shale partings.

A16. The Lockport Group is a thick (75 to 200 feet, east to west) sequence of dolostones and sandy dolostones with lesser amounts of limestone. The Lockport rocks are extremely resistant and form the Niagara or Lockport Escarpment, a major topographic feature extending from Rochester to the Niagara Gorge. East of the Rome meridian, the Lockport passes into the Ilion Formation of shales with interbedded dolostones.

Cayugan Series (Upper Silurian)

A17. Rocks of Late Silurian age belong essentially to the Salina and Bertie Groups. Exposures of the Salina Group are relatively rare as it is composed primarily of weak shales and was, therefore, usually stripped by glacial erosion. The Bertie rocks are more resistant carbonates, and therefore overlying glacial deposits are thin. The Salina Group is about 300 feet thick near Buffalo, increasing to about 950 to 1000 feet near Syracuse, thereafter thinning rapidly to a feathered edge near Richfield

Springs in Otsego County. Three formations are included in the Salina Group: Vernon Shale, salt of the Syracuse Formation, and the Camillus Formation.

A18. The Vernon Shale is composed of variegated red and green shale and siltstone totalling 300 to 400 feet in thickness. The base of the Vernon is Late Niagaran in age in eastern and central New York, and becomes progressively younger westward.

A19. The Syracuse Formation consists of three to four hundred feet of shale, dolostone and anhydrite which thicken rapidly southward to about 1,300 feet beneath Schuyler and Chemung Counties. With distance to the south, numerous salt beds occur, some reaching a maximum thickness of 400 feet. These salt beds rarely, if ever, outcrop at the surface, but the updip limit of the major salt beds is nearest the surface at Syracuse. This limit then strikes southwestward across the state, meaning that salt lies considerably deeper below the Genesee Valley than below Syracuse.

A20. Overlying the Syracuse Formation are 100 to 200 feet of shales, dolostones, and shaley dolostones of the Camillus Formation, containing a large quantity of interbedded anhydrite and gypsum.

A21. The remainder of the Cayugan Series above the Salina Group is composed of the Bertie Group dolostones (50 feet), Akron or Cobleskill limestones and dolostone (10 feet), and lower Chrysler Dolostone (30 feet). The Silurian-Devonian boundary occurs in the middle of the Chrysler Formation. Caverns, caves and other solution features are also common in these units.

Devonian System

A22. Devonian rocks are by far the most extensively exposed in New York State. Devonian strata underlie all of the Allegheny Plateau and regions adjacent to it, such as the Lake Erie Plain, southern Lake Ontario

Plain, and westernmost Hudson Valley Province. In addition, the beds are generally structurally simple, and the abundant fossil fauna in such rocks have led to a detailed stratigraphic description over the last fifty years.

A23. Carbonates dominate the Lower Devonian and lower part of the Middle Devonian while clastic deposits predominate in the remainder of the section. The clastics are fine-textured marine rocks at the bottom, grading upward to coarse, red continental sandstones. These deposits, are known as the Catskill Delta, and are a wedge of clastic sediments that thicken and become coarser to the east. The New York Devonian is subdivided into the Ulsterian Series (Lower Devonian), Erian Series (Middle Devonian) and Senecan and Chautauquan Series (Upper Devonian).

Ulsterian Series (Lower Devonian)

A24. Lower Devonian exposures are confined to the area west of Seneca Lake, along the north edge of the Allegheny Plateau and south along the north edge of the Allegheny Plateau and south along its eastern perimeter to Port Jervis. This largely carbonate sequence is divided into the Helderberg Group below and the Tristates Group above.

A25. The Helderberg Group is a complex of intertonguing carbonate facies. Lagoonal dolostones with minor evaporites pass eastward into an offshore shelf facies dominated by limestones, some with reefs. The Helderberg Group is about 240 feet thick in the Helderberg Mountains southwest of Albany, increasing southward to some 450 feet at Port Jervis near the New Jersey border. Westward the group thins to about 100 feet near Syracuse and is absent west of Geneva. Solution features and caverns (e.g. Howe Caverns) are common in the Helderberg Group.

A26. The Helderberg Group is succeeded in an upward direction by the Tristates Group of which the Oriskany Sandstone is the lowest unit. This formation is, in turn, succeeded by the Esopus Shale, Carlisle Center

AD-A169 701

REPORT ON SEISMIC STABILITY ONONDAGA DAM NEW YORK
ATTACHMENT NUMBER 2 (U) CORPS OF ENGINEERS BUFFALO NY
BUFFALO DISTRICT T X GRASSO ET AL. MAY 86

2/8

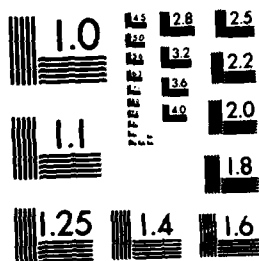
UNCLASSIFIED

DACW49-81-D-0011

F/G 13/2

ML

END
13 50



MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS-1963-A

Shale and Schoharie Limestone in eastern New York only. The Tristates Group is about 160 feet thick in the Albany region, thickening southward to about 750 feet at Port Jervis, and except for isolated patches of Oriskany, disappears west of Richfield Springs.

A27. The Oriskany Sandstone is the most important formation in the Tristates Group. It underlies much of the Allegheny Plateau, except in the southwest part of the state, and is 0 to 50 feet of quartzose sandstone, with calcite and silica cement. This sandstone is porous and permeable in siliceous zones and tight where calcareous. In the west, it is a prolific gas producer. Eastward in the Allegheny Plateau, there is no known gas production in the Oriskany Sandstone.

Erian Series (Middle Devonian)

A28. Two stratigraphic units make up the Middle Devonian in New York State, the Onondaga Limestone and the Hamilton Group. They are exposed across the entire state, in a continuous belt, from Lake Erie on the west to the New Jersey border near Port Jervis in the southeast.

A29. The Onondaga Limestone is a regional, blanket carbonate deposit, thinning from about 175 feet at Buffalo to only some 70 feet at Syracuse, but thickens eastward again to about 250 feet at Catskill and 300 feet at Port Jervis. It is a thin to medium bedded, medium to coarse textured, gray limestone, often containing bedded chert and chert nodules. Onondaga rocks are extensively quarried across the state.

A30. The Hamilton Group is a clastic wedge (Catskill Delta) deposited by westward flowing streams from a rising mountainous region to the east and southeast of the present Catskill region. The group is about 265 feet thick in western New York, 1,100 feet in central New York and 2,500 to 3,000 feet thick in the Catskills. At the toe of the delta complex, in an ocean basin in what is now western New York, fine-grained, bituminous, dark to black Hamilton shales were deposited. Uppermost Hamilton beds in western New York are gray calcareous shales. The shales pass eastward

into coarse, silty shales and sandstones. These, in turn, grade eastward and interfinger with mostly terrestrial, crossbedded, red and green sandstones, shales and siltstones.

A31. Three thin but persistent carbonate units (key beds) pierce the clastic wedge and were deposited over the delta at times of low sediment influx. These serve to subdivide the Hamilton into four formations, the Marcellus, Skaneateles, Ludlowville, and Moscow. The carbonate beds are from oldest to youngest, Mottville Member, Centerfield Member and Portland Point Member.

Combined Senecan and Chautauquan Series (Upper Devonian)

A32. The Upper Devonian sequence is a continuation of the Catskill Delta which was migrating westward during deposition. The same general Middle Devonian facies continue but persistent key beds are black shale tongues, not carbonate units, except for the lowest formation, the Tully Limestone. The black shales subdivide the Upper Devonian into a number of groups, totalling some 3,300 feet of thickness in southwest New York, 7,000 feet in south central New York, and 7,700 feet in the Catskills. The Upper Devonian is exposed throughout the southern tier region of the Allegheny Plateau, however the Chataquan Series is present only in the western part, as it has been removed further eastward by erosion.

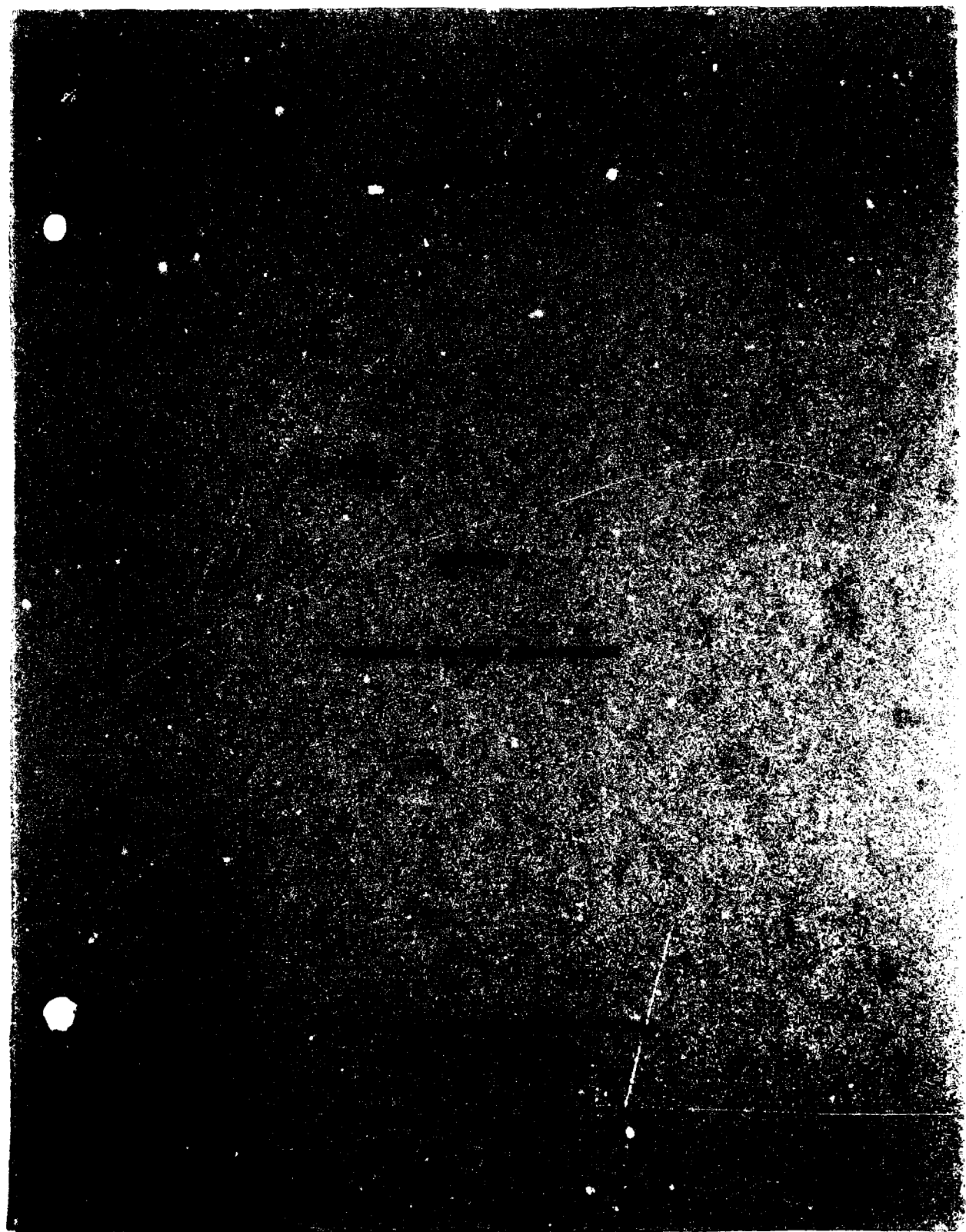
Senecan Series

A33. The Tully Limestone is the basal formation of the Senecan Series. It reaches a maximum thickness of 100 feet in east central New York and changes eastward into the Gilboa Shale and sandstone. The Tully is approximately 30 feet thick in the Syracuse region and is not present west of Canandaigua Lake due to its being removed by erosion prior to deposition of overlying units.

A34. Overlying the Tully is the Genesee, Sonyea and West Falls Groups. They are black shales in the west and red beds in the east, although each facies is found further westward in the West Falls Group.

Chautauquan Series

A35. The Chautauquan Series is made up of the Canadaway, Conneaut, and Conewango Groups. The Canadaway and Conneaut Groups contain several fresh water aquifers as well as the major oil producing sands of SW New York. They have been penetrated by numerous borings but records of many of the early wells have now been lost. Much of the oil is trapped along axis of the broad anticlinal structures that characterize the southwest portion of the Allegheny Plateau. If salt water is not present, the oil may also be found along the synclinal axes.



APPENDIX B
ONONDAGA DAM SITE STRATIGRAPHY

B1. Older Silurian units are exposed to the north of the dam in Syracuse and younger Upper Devonian units cap the hills south of the dam, and form the valley sides and floor south of the Village of Tully.

B2. Table III presents the detailed stratigraphic column along the Syracuse Meridian in the vicinity of the dam. Figure 4 is a geologic map of the area surrounding the dam. A description of the stratigraphy summarized on Table III and Figure 4 follows.

Lower Devonian - Ulsterian Series

Helderberg Group

B3. The Helderberg Group in the Onondaga Valley region is composed of two formations which, in ascending order, are the Rondout Formation (Chrysler Dolostone) and the Manilus Formation (Table III). The Rondout is poorly exposed in this area, however, a limited outcrop is exposed in the base of the rock cuts on Interstate 81, just south of the junction with Interstate 481.

B4. The Manilus Formation is subdivided from older to younger, into the Olney Limestone Member, Elmwood A, B and C Dolostone Member, Clark Reservation Limestone Member and the Jamesville Limestone Member.

- a. Olney Member - Named for Olney Station on the now abandoned Auburn and Syracuse Electric Railroad, is approximately 30 feet of fine-grained, even bedded limestones. The lower portion is thin-bedded; the upper portion is somewhat more thickly bedded, although both may have laminated beds. Fossils are neither abundant nor well diversified. The Olney underlies the spillway cut at the east end of the dam and is exposed in road cuts on Interstate 81 south of the junction with Interstate 481.

- b. Elmwood Member - Overlies the Olney Member and consists of a series of dolostones, waterlimes, and limestones. The name is derived from Elmwood Park near Onondaga Hill in the southwestern part of Syracuse. It is subdivided into 3 units known as Elmwood A, B, and C in ascending order. Elmwood A is a drab yellowish brown, thin-bedded and mud cracked waterlime, barren of fossils and about 6 feet thick. Elmwood B is a fine-grained, blue limestone, 3 feet thick, locally known as the "Diamond blue". This is overlain by 4 feet of waterlime, similar to Elmwood A, and known as Elmwood C. The Elmwood is exposed on the west side of the Onondaga Valley in Sweet's Quarry and Split Rock Quarry, in the roadcuts along Interstate 81 south of Interstate 481 junction, at Clark Reservation State Park, and in the vicinity of the Village of Jamesville. It underlies the spillway rock cut at the east edge of the flood control structure.
- c. Clark Reservation Limestone Member - Lies above the Elmwood and consists of a bed of fine-grained, dark blue, white-weathering limestone characterized by a diagonal fracture system. The name comes from Clark Reservation State Park, one mile west of Jamesville, where the type section is located. It is approximately 5 feet thick and in addition to the type locality it is exposed at Onondaga Hill on the west side of the valley and in the rock cuts along Interstate 81 mentioned above. It is also exposed at the base of the exposure on the east side of U. S. 11 just north of the Nedrow interchange to Interstate 81.
- d. Jamesville Limestone Member - Lies between the Clark Reservation and the Onondaga Limestone. It is approximately 25 feet of fine-grained, dark blue, brittle,

limestone in thin, even beds at its type exposure in Clark Reservation State Park. It makes up the lower 12 feet of the rock exposure in the spillway cut at the east edge of the dam. The lowest 2.5 feet of the Jamesville exposed at this location is a stromatoporoid biostrome which is a layer composed of enormous numbers of stromatoporoids, perhaps a type of fossil sponge. A complete section of the Jamesville is exposed on U.S. 11 just north of the Nedrow interchange to Interstate 81, and in the rock cuts on Interstate 81 south of the junction of Interstate 481.

Tristates Group

B5. The Oriskany Sandstone, the only formation of this group exposed in central New York, is not well developed in the vicinity of the dam and Onondaga Valley in general. A few inches of reddish-orange, sandy layers at the base of the Onondaga Limestone represent this unit in the spillway rock cut.

Middle Devonian - Erian Series

Onondaga Limestone

B6. The Middle Devonian sequence begins with the Onondaga Limestone which attains a thickness of approximately 70 feet in the Syracuse area. It is a series of light bluish-gray semi-crystalline limestones in even continuous layers from 1 inch to 2 feet in thickness, separated by thin seams of dark, calcareous shale. Flattened nodules of dark blue or black chert, sometimes in continuous sheets or beds are unevenly distributed throughout the Onondaga but mainly in the upper part.

B7. The Onondaga is comprised of five members as shown on Table III and discussed below:

- a. Edgecliff Member - The lowest member of the Onondaga is distinguished by its coarser, more pronounced, crystalline texture and abundant fossils. The base of the Edgecliff is a disconformity, meaning it was deposited over an erosion surface that had removed previously deposited rock strata. The member is approximately 15 ft. thick and is exposed in the spillway cut.
- b. Nedrow Member - Just above the Edgecliff, is characterized by thin-bedded, shaley limestone, marked in the lower part by abundant snail fossils. It is a medium gray, very fine-grained, argillaceous limestone containing little chert unevenly distributed at the top. It is approximately 10 ft. thick, is gradational with the overlying Moorehouse Member, and rests with a sharp contact on the Edgecliff. Both the Nedrow and the overlying Moorehouse Member occur in the spillway cut.
- c. The Moorehouse Member - Consists of 20 to 25 feet of medium gray, very fine grained limestone beds which are separated in many places by thin shaley partings ranging from 2 inches to 5 feet thick. Dark gray chert is common in this member but varies in amount. It is more common in the upper part of the member and occurs as more or less continuous strata.
- d. Tioga Bentonite Member - Occurs in the upper part of the Onondaga as a volcanic ash layer, 6 to 9 inches thick. It separates the underlying Moorehouse Limestone Member from the overlying Seneca Limestone Member. Neither the Tioga nor the Seneca Member were observed in the spillway cut.
- e. Seneca Member - Although lithologically similar to the upper Moorehouse, is characterized by a slightly different fossil assemblage. Also chert is less common in the upper part of the Seneca Member and the limestone beds are darker gray. This Member is approximately 25 feet thick.

B8. The Onondaga Limestone is exposed in the upper 50 feet of the spillway cut on the east side of the dam. Other exposures in the vicinity of the dam include Hemlock Creek northeast of Indian Village, abandoned quarries along Quarry Road on the Indian Reservation, road cuts on U.S. 11 at the Nedrow interchange to Interstate 81, road cuts on Interstate 81 north of the Nedrow exit, along the west branch of Onondaga Creek 3 miles northwest of South Onondaga and near Cedarvale. Additional exposures occur along the northern margin of the Allegheny Plateau from Jamesville through southern Syracuse and the famous quarries at Split Rock.

Hamilton Group

B9. The Hamilton Group in the Tully Valley consists of approximately 1,100 feet of mostly silty shales and siltstones lying above the Onondaga Limestone and below the Tully Limestone. Three thin carbonate beds serve to define the formational boundaries. The succession of units, their lithologies and thicknesses are shown on Table III.

B10. The Hamilton is the rock unit exposed in the dam vicinity except for the spillway cut at the dam itself. The Hamilton forms the sides of the Onondaga Valley north and south of the dam and also the floor of the valley south of the dam (Figures 3 and 4).

B11. The Hamilton rocks in the Tully Valley dip $S25^{\circ}W$ at approximately 48 feet per mile, based upon the Centerfield Member as a datum. Formations included in the Hamilton Group are described below, in ascending order.

Marcellus Formation

B12. The Marcellus Formation is divided into four members. The lower three are composed of black shales and limestone, while the upper member is dominantly a transition from black to gray shale. The lower contact of this formation with the Onondaga Limestone is fairly

sharp throughout the area and is overlain by the Mottville Member of the Skaneateles Formation with a sharp contact.

- a. Union Springs Member - In the vicinity of the dam, the Union Springs Member is 10 feet thick. It is an alternation of black, sooty, carbonaceous, calcareous shale with thin bands and lenses of black bituminous limestone. The limestone beds range from less than 1 inch to 6 inches in thickness. Pyrite is disseminated throughout the Union Springs, many fossils being replaced by it. The contact with the overlying Cherry Valley Member is sharp.
- b. Cherry Valley Member - The Cherry Valley Limestone is a hackly, black, bituminous limestone with an orange-red stain that fills cavities, stains the surface, and coats some of the fossils. It is approximately 3 ft. thick.
- c. Chittenango Member - This member consists of about 80 feet of unfossiliferous, jet black, non-calcareous, bituminous shale that contains abundant pyrite. It is extremely thin bedded and displays excellent rectangular jointing. Wherever it is exposed it breaks into paper-thin flakes that are usually stained a rusty color. Calcareous, unfossiliferous, septarian concretions up to 6 or more feet in diameter are common in this member, as are also smaller non-septarian concretions. These shales grade upward into the overlying Cardiff.
- d. Cardiff Member - This member is named for exposures around the Village of Cardiff in the Tully Valley. It is a non-calcareous, dark gray to grayish black, medium-bedded shale at its base and grades upward into a slightly coarser shale of medium dark gray color. Toward the top, particularly the uppermost 4 feet, small, round, calcareous, non-septarian, unfossiliferous concretions are present. They range from 1 inch to 8 inches in diameter. As defined here,

the Cardiff is approximately 180 feet thick and is overlain by the Mottville Member of the Skaneateles Formation.

B13. The Marcellus Formation is exposed in the immediate vicinity of the dam. It makes up the lower slopes of the valley north of the dam, and south of the dam to a point south of Rattlesnake Gulf (Bare Mountain Ravine) approximately 6 miles south of dam. It is also exposed on the lower slopes of the valley along the west branch of Onondaga Creek westward through Pumpkin Hollow to the Village of Marcellus, the type section. Streams cascading down the sides of the Tully Valley notch ravines in which the Marcellus Formation is exposed in the lower reaches. Peppermill Gulf and Lords Hill Ravine near Ironsides are two excellent exposures, as well as ravines flowing down the east side of the valley south of the dam. Road cuts on Route 11A south of the dam expose Marcellus to a point a few miles south of Cardiff. The lower part of the formation, the Union Springs and Cherry Valley Limestone Members, are best exposed north of the dam. Two localities would be Hemlock Creek and a road cut on Interstate 81 just north of the Nedrow exit on the east side of the highway.

Skaneateles Formation

B14. In the Tully Valley, the Skaneateles Formation, with the exception of the Mottville Member, represents the intertonguing of a black shale facies to the west with a silty shale facies more fully developed to the east. Three tongues of dark shale, which are the eastern elongations of the Levanna Member of Cayuga Lake, extend into the silty facies and thereby provide the basis for subdividing this formation, above the Mottville, into three members. Each member begins with a dark, relatively unfossiliferous shale that passes upward into coarser, lighter colored, more fossiliferous beds. These are in turn sharply overlain by the next dark shale of the member above.

- a. Mottville Member - In the Tully Valley, the Mottville Member is about 5 to 6 feet thick. It is highly fossiliferous, calcareous, medium gray shale containing one or two thin limestone beds.

- b. Delphi Station Member - At most localities this member is about 125 feet thick, except at Rainbow Creek where 145 feet is measured. It is a dark gray shale in its lower portions containing abundant irregularly shaped, calcareous, non-septarian, unfossiliferous concretions from 1 to 5 inches in diameter. The dark shales pass upward into slightly calcareous siltstones abounding in bivalves (clams). The contact with the overlying Pompey is sharp.
- c. Pompey Member - The Pompey Member is 60 feet thick, with the exception of the exposure in Bare Mountain Ravine, where it is about 114 feet thick, probably due to faulting. It is a dark gray to grayish black shale at its base grading upward to silty shale and finally siltstone at the top. Concretions occur more or less throughout the section, but are especially conspicuous in the upper 5 feet. Most of them are small, rounded, non-septarian and calcareous, and less than 4 or 5 inches in diameter. Although some fossils may be found on the periphery of the concretions, few are fossiliferous internally. The upper 20 feet of the Pompey contains abundant pyrite as nodules and disseminated particles
- d. Butternut Member - The characteristic feature of the lower and middle Butternut is that it is a "paper shale", i.e., it splits into thin flakes, a distinct change from the underlying Pompey Member. Upward, this lithology is gradually replaced by silty shales interbedded with thin siltstone bands and lenses. The lenses vary in size from 3 feet long with maximum thickness of 1 inch, to 24 feet long with a maximum thickness of 6 inches. The lenses are cross-bedded at their bases only. Higher in the section, the silty shale gradually gives way to siltstone and the uppermost 4 or 5 feet of the Butternut is

a calcareous flaggy siltstone. The contact with the overlying Centerfield Member is transitional and difficult to place. The Member is approximately 135 feet thick.

B15. The Skaneateles formation is exposed in virtually all the ravines mentioned above for the Marcellus, only in the middle and upper reaches of these ravines, usually above the first waterfall, which is held up by the Mottville Member. Therefore, it forms the middle slopes of the valley sides in the vicinity of the dam.

Ludlowville Formation

B16. The Ludlowville Formation is the most fossiliferous formation in the Tully Valley. It consists of intertonguing silty shales and siltstones and, on the basis of these, can be divided, above the Centerfield Member, into four members.

- a. Centerfield Member - In the Tully Valley area, the Centerfield Member is a coarse siltstone about 25 feet thick. The lower and upper 10 feet are flaggy but the middle portion is massive. Only the middle portion is highly calcareous and fossiliferous. The siltstone beds in the flaggy parts are about 1 inch thick and at some localities crossbedded. The contact with the overlying Otisco is sharp.
- b. Otisco Member - This unit is a soft, thinly bedded, slightly calcareous, silty, medium-gray to medium-dark-gray shale, which is interbedded toward the top with thin siltstone beds. The contact with the Ivy Point Member, about 165 feet above the Centerfield, is sharp. This member is especially interesting for two coral biostromes which have been given submember status by Oliver (1951). The lower, designated the Staghorn Point Submember, is about 7 feet thick in the Tully Valley. The Staghorn Point occurs about 50 feet above the top of the Centerfield and rests on a massive

calcareous siltstone platform about 3 feet thick. The upper biostrome, named the Joshua Submember, varies from 0 to 55 feet in thickness in the Tully Valley. It covers some 40 square miles, less extensive than the Staghorn Point Submember.

- c. Ivy Point Member - This member is about 36 to 37 feet thick and is dominantly a flaggy, slightly calcareous, gray siltstone, that weathers to a distinct yellowish-brown color. Most of the siltstone beds are approximately 1 inch thick. Spheriodal, calcareous, nonseptarian, unfossiliferous concretions 3 to 18 inches in diameter are present in the lower 20 feet.
- d. Spafford Member - The Spafford Member consists of 27 feet of thin-bedded, gray to dark gray shale and silty shale sharply overlying the coarse Ivy Point Member. Although the Spafford becomes coarser upward, the upper 3 feet are very fine-grained.
- e. Owasco Member - This is a calcareous, massive siltstone bed 2 feet thick.

B17. The Ludlowville Formation makes up the upper slopes of the Tully Valley and is best exposed in Lords Hill Ravine, and in Emerson Gulf and Fellow Falls Ravines south of Bare Mountain Ravine.

Moscow Formation

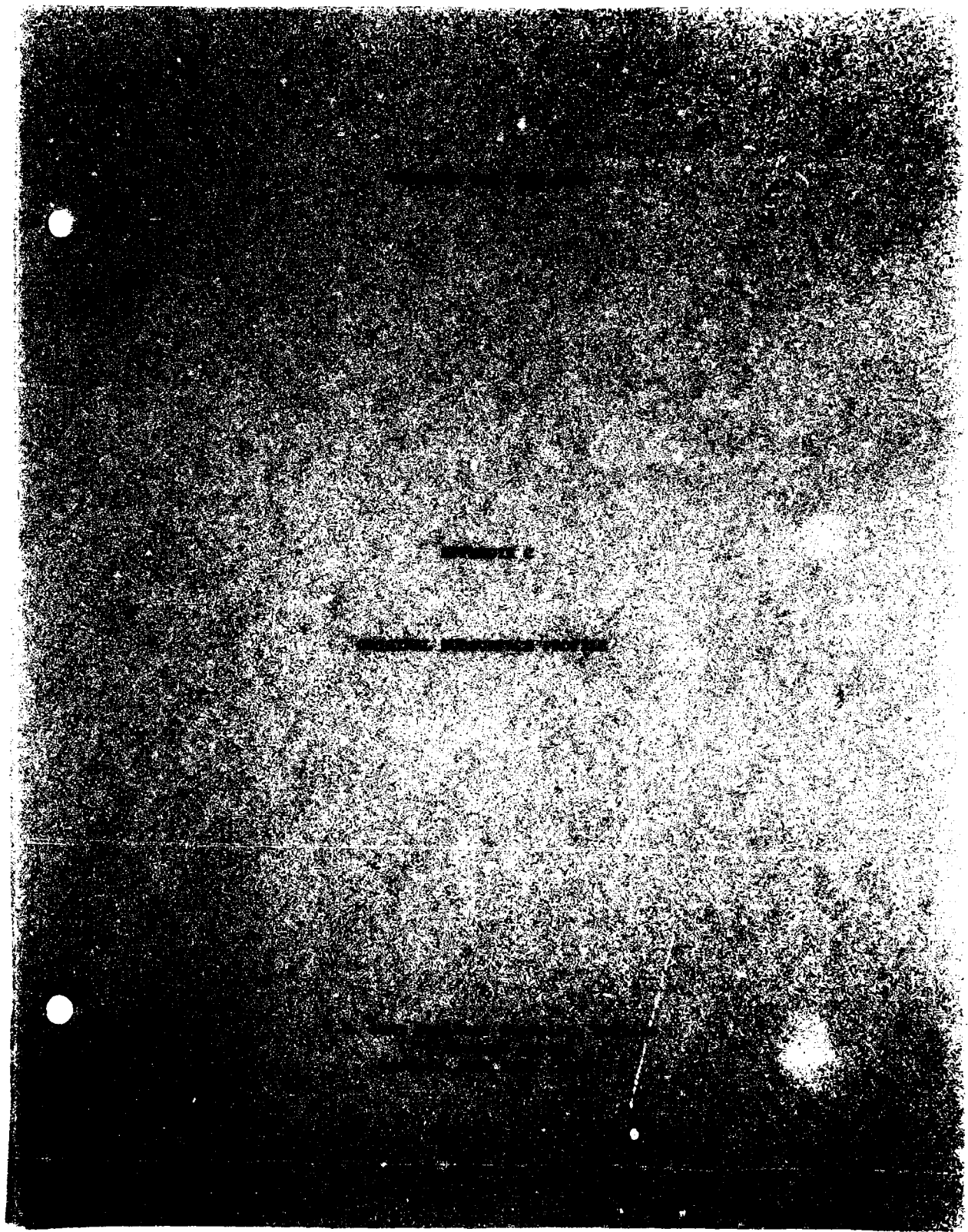
B18. Exposures of the lower 70 feet of the Moscow Formation are difficult to find in the Tully Valley. Even where this portion crops out in a ravine, the exposure is poor and incomplete. Therefore, this interval could not be examined thoroughly and much of the remarks pertaining to the Portland Point and lower and middle Windom Members were derived from the study of a single section. The upper 80 feet of this formation is well exposed as it occurs in high waterfalls capped by the resistant Upper Devonian Tully Limestone.

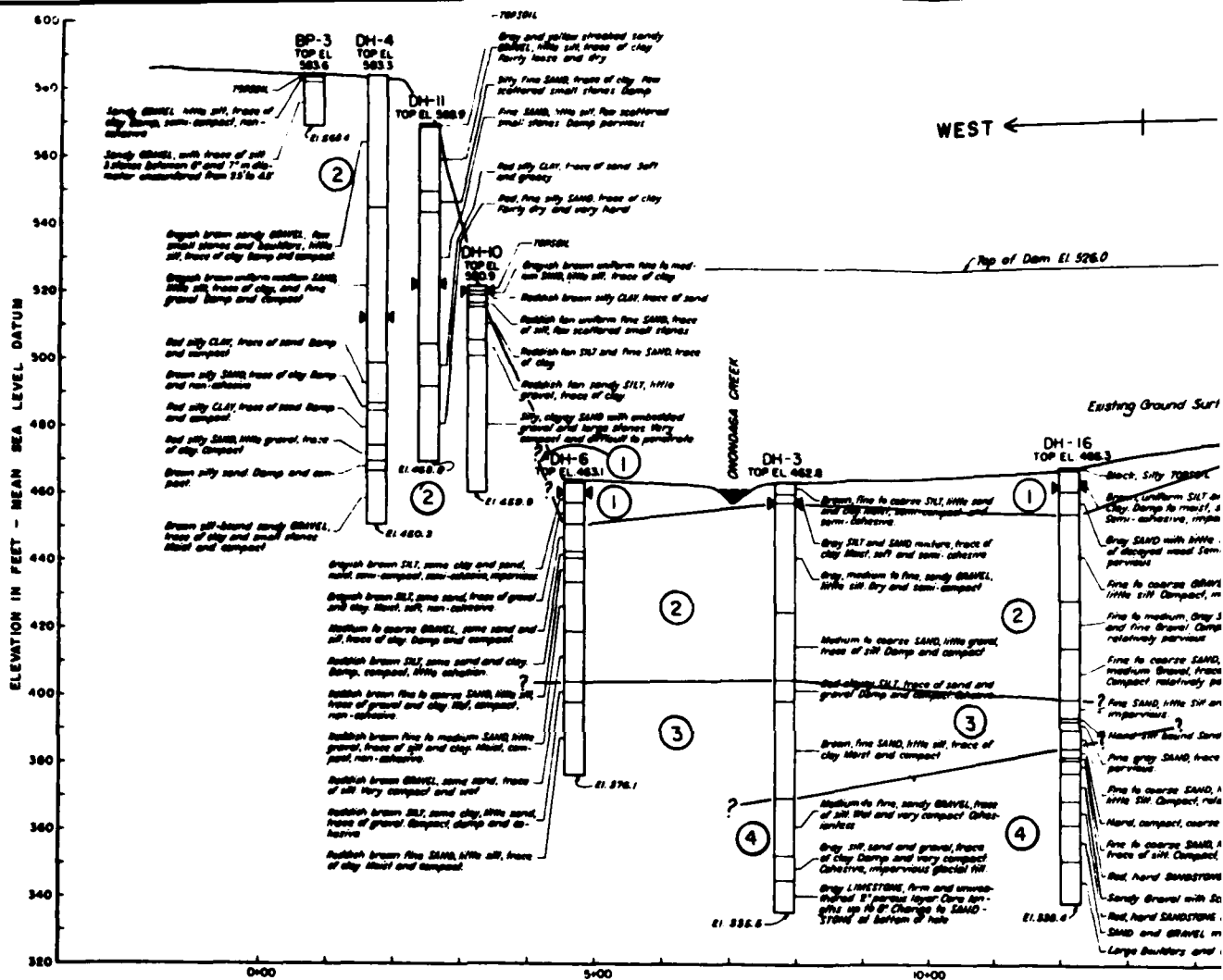
- a. Portland Point Member - The upper and lower contacts of the Portland Point Member are sharp. A basal crinoidal, shelly limestone about 1 foot thick is succeeded by 11 feet of gray, silty shale, interbedded with thin, crinoidal, shelly limestone bands 2 inches thick or less and about 8 inches apart. The silty shales are not very fossiliferous.
- b. Windom Member - The Windom Member is a thin-bedded, gray to medium-gray shale grading upward to medium-gray silty shale to a point 20 feet below the Tully. Here a sharp lithologic change to a medium-gray to dark gray or grayish black, non-calcareous, pyritiferous shale takes place and this is in turn sharply overlain by the Tully Limestone. This dark shale appears as a reddish orange zone beneath the Tully due to the weathering of the pyrite.

B19. The Windom is 163 feet thick in the Tully Valley. The Moscow Formation is exposed at the tops of the hills south of U.S. Route 20 such as Bare Mountain. It is well exposed in the vicinity of the Village of Tully in ravines and road cuts.

Upper Devonian - Senecan & Chautauquan Series

B20. The Upper Devonian sequence begins with the Tully Limestone exposed at the tops of the highest hills, at about El. 1,600 feet just south of the dam such as at Bare Mountain. However, the well developed and best exposed Upper Devonian does not occur until south of the village of Tully. The Upper Devonian is judged to be relatively far removed from the site and therefore no details of the stratigraphy will be given other than those shown on Table II and in Appendix A.





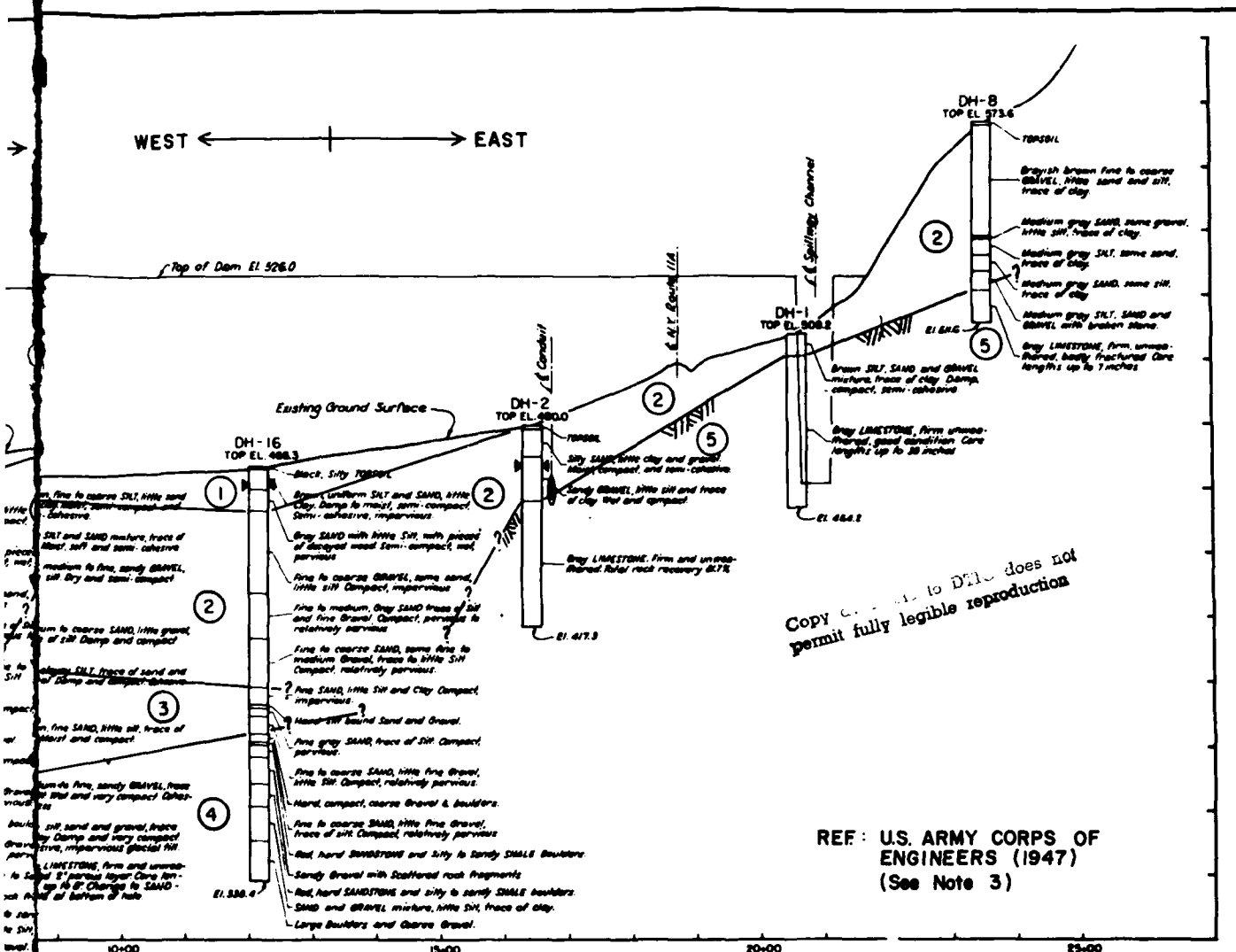
PROFILE ALONG & DA

GEOLOGIC LEGEND:

STRATUM NO.	GENERALIZED GEOLOGIC DESCRIPTION
①	FLUVIAL OVERBANK
②	DELTAIC
③	LACUSTRINE
④	GLACIAL TILL
⑤	BEDROCK

NOTES:

1. STRATA LINES ARE GR. FROM SUBSURFACE CO LOCATIONS. ACTUAL C MAY VARY. QUESTION OF UNCERTAINTY.
2. SEE TEXT FOR DISCUS:
3. INDIVIDUAL SOIL DESCRIP CORP OF ENGINEERS PLA STRATA LINES AND GEOLO OF NEW YORK, MARCH 19



PROFILE ALONG & DAM

NOTES:

1. STRATA LINES ARE GRADATIONAL AND INTERPOLATED FROM SUBSURFACE CONDITIONS AT EXPLORATION LOCATIONS. ACTUAL CONDITIONS BETWEEN EXPLORATIONS MAY VARY. QUESTION MARKS INDICATE A HIGH DEGREE OF UNCERTAINTY.
2. SEE TEXT FOR DISCUSSION OF SUBSURFACE CONDITIONS.
3. INDIVIDUAL SOIL DESCRIPTIONS OBTAINED FROM U.S. ARMY CORP OF ENGINEERS PLANS FOR ONONDAGA DAM DATED 1947. STRATA LINES AND GEOLOGIC DESCRIPTIONS ADDED BY H&A OF NEW YORK, MARCH 1982.

ONONDAGA DAM, NEW YORK

PROFILE ALONG DAM CENTERLINE

U.S. ARMY ENGINEER DISTRICT BUFFALO
TO ACCOMPANY PHASE I, SEISMIC STABILITY INVESTIGATION
DATED: AUGUST 1982

FIGURE C-1

APPENDIX B

THESE DOCUMENTS ARE 100-101 THROUGH 100-103

DRILLING LOG (Cont Sheet)		REVISION TOP OF HOLE		Hole No.		
PROJECT		525.8 ft NSL		D82-101		
ONONDAGA DAM		BUFFALO DISTRICT		SHEET 2 OF 9 SHEETS		
ELEVATION	DEPTH	LOGGING	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)
	10	SET	Gravelly coarse to fine SAND	10.0	10.0	ADVANCE BORING w/ 3.75 IN ID HOLLOW STEM AUGER MFG. CME
	11	13	little silt, medium dense damp, brown	B	JAR 7	DRIVE 2.0 ft REC 1.0 ft LOSS 0.0 ft
	12	17	SM	A		3.0x3.5x38 SPECT SPOON DRIVEN w/ 350 lb HAMMER
	12	28		G	12.0	ADVANCE BORING TO 15.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG. MFG. CME
	13			5	NS	
	14					
	15	SET	Gravelly coarse to fine SAND, little silt, very dense, damp, brown	15.0		DRIVE 1.2 ft REC 1.2 ft LOSS 0.0 ft
	16	38	SM	JAR 8	16.2	NOTE LARGE GRAVEL PARTICLE IN NOSE OF SPOON
	17	50		B		ADVANCE BORING TO 20.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG. MFG. CME
	18			A		
	19			G	NS	
	20	SET	Gravelly coarse to fine SAND	20.0		DRIVE 0.9 ft REC 0.9 ft LOSS 0.0 ft
	21	50	little silt, very dense damp, brown	JAR 9	20.9	
	22		SM	NS		

ENG FORM 1836-A

DATE 10-20-60

PROJECT
ONONDAGA DAM

HOLE NO.
D82-101

DRILLING LOG (Cont Sheet)		ELEVATION TOP OF HOLE		Hole No.		
PROJECT		INSTALLATION		DATE		
ONONDAGA DAM		BUFFALO DISTRICT		1982-101		
ELEVATION	DEPTH	LOGGING	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	LOG USE SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)
503.3	22		Coarse to fine SAND, fine silt, very dense, damp brown - SM	22.5		ADVANCE BORING TO 25 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG - CME
	23				NS	SAMPLE CONDUCTED w/ 3.0 x 3.5 x 38 IN TEST SPOT DRIVEN BY 350 LB HAMMER FALLING FREE 18 IN
	24					
	25	SET	Fine to med. coarse SAND, trace of gravel, fine silt, very compact, damp, brown	25.0	25.0	DRIVE 0.5 ft REC 0.9 ft LOSS 0.0 ft
	26	50	SP	25.9	25.9	
	27				NS	DRILLER NOTES MATERIAL LESS DENSE @ APPROX 27 ft. AT APPROX 28.5 ft MATERIAL BECOMES MORE DENSE.
477.3	28			28.5		ADVANCE BORING TO 30.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG - CME
	29					DRILLER NOTES HARD MATERIAL ENCOUNTERED FROM APPROX 29 to 30 ft
	30	SET	Coarse to fine sandy GRAVEL with silt, very dense, damp brown - GM	30.0	30.0	DRIVE 1.3 ft REC 1.3 ft LOSS 0.0 ft
	31	36		31.3		
	32	50				
	33					
	34					

SDS FORM 1000-A

SDS FORM 1000-A

ONONDAGA DAM

1982-101

DRILLING LOG (Cont Sheet)		REVISION TOP OF HOLE		525.8 ft MSL		Hole No. DB2-101	
PROJECT		SUBPROJECT		RUFFALO DISTRICT		PAGE 4 OF 4	
DEPTH	LOGGING	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of penetration, etc., if significant)		
34					ADVANCE BORING TO 35.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG. CME		
35	SET	Coarse to fine sandy GRAVEL, little silt, very dense, damp, brown		35.0			
36	45	GM		JAR 12	DRIVE 1.4 ft REC 1.4 ft LOSS 0.0 ft		
37	30			36.4			
38					ADVANCE BORING TO 40.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG. CME		
39				NS	SAMPLING CONDUCTED w/ 3.0 x 3.5 x 38 IN SPLIT SPOON DRIVEN BY 350 LB HAMMER FALLING FREE 18 IN. MATL. APPEARS TO BE MORE DENSE AT APPROX 37 ft.		
40	SET	Coarse to fine sandy GRAVEL, little silt, very dense damp, brown		40.0			
41	30	GM		JAR 13	DRIVE 0.7 ft REC 0.7 ft LOSS 0.0 ft		
42				40.7			
43					MATL APPEARS TO BE MORE DENSE AT APPROX 42.5 ft		
44				NS	ADVANCE BORING TO 45.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG. CME		
45	SET	Gravelly, coarse to fine SAND, little silt, very dense, damp, brown		45.0			
46	44	SM		JAR 14	DRIVE 1.3 ft REC 1.3 ft LOSS 0.0 ft		

SDS FORM 1886-A

DATE: 1988-07-10

PROJECT: ONONDAGA DAM HOLE NO: DB2-101

DRILLING LOG (Cont Sheet)		REVISION TOP OF HOLE		Hole No. DB2-101		
PROJECT		INSTALLATION		SHEET		
ONONDAGA DAM		BUFFALO DISTRICT		29		
ELEVATION	DEPTH	LOGGING	CLASSIFICATION OF MATERIALS (Description)	S CODE RECOV. BY	LOG OF SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)
46	50		Generally coarse to fine SAND, little silt, very dense, damp, brown SM		46.3	ADVANCE BORING TO 50.0 FT W/ 3.75 IN ID HOLLOW STEM AUGER W/ PLUG MFG. CME SAMPLING CONDUCTED BY 30X35X35 IN DRIVEN W/ 350 LB HAMMER, FALLING FREE 18 IN.
47				B		
48				A	NS	
49				G		
50				8		30 JUL 82 AM DEPTH TO BOTTOM OF CSG 50.0 FT DEPTH TO HOLE 46.8 FT
51	30	SET	Generally coarse to fine SAND, little silt, very dense, moist, brown SM		JAR 15	DRIVE 2.0 ft REC 1.7 ft LOSS 0.3 ft
52	37					
53	45			53.0	52.0	
54					NS	ADVANCE BORING TO 55.0 FT W/ 3.75 IN ID HOLLOW STEM AUGER W/ PLUG MFG. CME
55				55.0	55.0	
56	10	SET	Fine sandy SILT, little clay, medium dense, moist, brown, ML	B A G 9	JAR K	DRIVE 2.0 ft REC 1.8 ft LOSS 0.2 ft
57	13			57.0	57.0	
58					NS	ADVANCE BORING TO 60.0 FT W/ 3.75 IN ID HOLLOW STEM AUGER W/ PLUG MFG. CME

SDS FORM 1834-A

SDS 1000 01-100-000

PROJECT

ONONDAGA DAM

HOLE NO.

DB2-101

DRILLING LOG (Core Sheet)		ELEVATION TOP OF HOLE		Hole No.	
PROJECT		INSTALLATION		SHEET	
JONADAGA DAM		BUFFALO DISTRICT		OF 9 SHEETS	
ELEVATION	DEPTH	LOGNO	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	REMARKS (Drilling time, water loss, depth of penetration, etc., if significant)
a	b	c	d	e	f
	58				ADVANCE BORING TO 60.0 ft w/3/8" ID HOLLOW STEM AUGER w/PLUG MFG CME
	59				DRIVE 1 3/8" x 2.0 x 2 7/8" IN STANDARD SPIT SPOON w/140 LB SAFETY HAMMER FALLING FREE 30 IN
465.8	60		60.0		
	60	SET	Fine sandy SILT & CLAY trace fine gravel & coarse to medium sand in layers, medium dense, moist, brown		DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft
	61	9		JAR	
	61	8		17	
	62	8	CL		
463.8	62		62.0		
	62	SET	Silty fine SAND, loose, moist, brown	JAR	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
463.3	63	9	SM	18	
	63	13	Silty CLAY, little fine sand, trace organics, hard, moist, brown	JAR	
	64	17	CL	19	
	64	SET	Silty CLAY, little fine sand, trace organics, hard, moist, brown		DRIVE 2.0 ft REC 1.7 ft LOSS 0.3 ft
	65	15	CL	JAR	
460.3	65		65.5	20	
	66	24	Silty CLAY, little coarse to fine sand, moist, brown	JAR	
459.8	66	SET	CL	21	
	67	17	Coarse to fine sandy GRAVEL, little silt, medium dense, moist, brown	JAR	DRIVE 2.0 ft REC 0.8 ft LOSS 1.2 ft
	67	13	GM	22	
	68	17			
	68	SET	Coarse to fine sandy GRAVEL, little silt, medium dense, moist, brown	B	DRIVE 3.0 x 3.5 x 38" SPIT SPOON WITH 3 x 2 1/2" MAX DIA CR FREE FALLING 18 IN
	69	15	GM	JAR	DRIVE 2.0 ft REC 1.8 ft LOSS 0.2 ft
	69	22		23	
	70	21		G	
	70			10	

SDS FORM 1836-A

SDS FORM 1836-A

PROJECT JONADAGA DAM

HOLES NO. DE1-101

DRILLING LOG (Cont Sheet)		ELEVATION TOP OF HOLE		PROJECT		HOLE NO.	
PROJECT		INSTALLATION		SHEET		SHEET	
ONONDAGA DAM		525.84 MSL		BUFFALO DISTRICT		D82-101	
ELEVATION	DEPTH	LOG NO.	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	SOIL OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)	
452.8	70	SET	Coarse to fine sandy GRAVEL, little silt, medium dense, wet, brown	B	70.0	DRIVE 3.0 x 3.5 x 38" SPLIT SPOON W/ 350 lb HAMMER FALLING FREE 18 IN	
	18			A			
	71	18	GM	G	24	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	
	25			10			
	72	25			72.0	72.0	
450.8	72	SET	Coarse to fine sandy GRAVEL, little silt, medium dense, wet, brown		JAR	DRIVE 2.0 ft REC 1.0 LOSS 1.0	
	19		GM		25	73.0	
	73	25	Medium to fine SAND, trace coarse sand, trace silt, dense, wet, brown		JAR	WATER INTRODUCED INTO BOREHOLE IN ORDER TO RETAIN HYDROSTATIC HEAD.	
	30		SP		26	74.0	
	74	30					
450.8	75	18			JAR	DRIVE 1 7/8 x 2.0 x 2 1/2" STANDARD SPLIT SPOON W/ 140 lb SAFETY HAMMER FALLING FREE 30 IN	
	21		Coarse to fine SAND, little gravel, little silt, dense, moist, brown		27	75.0	
	25		SM		JAR	DRIVE 2.0 ft REC 0.7 ft LOSS 1.3 ft	
	76	25			28	76.0	
	76	25					
449.8	77	SET	Coarse to fine sandy GRAVEL, trace silt, very dense, wet, brown	B	JAR	DRIVE 3.0 x 3.5 x 38" SPLIT SPOON W/ 350 lb HAMMER FALLING FREE 18 IN.	
	31		GW	A	29	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	
	29			G			
	78	32		11			
	78	32					
447.8	78	SET	Silty coarse to fine SAND, little gravel, very dense damp, brown		JAR	2 AUGUST 1992 AM DEPTH TO BOTTOM OF CSG 78.0 FT DEPTH TO WATER 47.0 FT CONTRACTOR NO. 105512 RD ONE 5/8" RACE HAMMER ROTARY PEG JEGAL & 14665 PULL AUGERS BACK TO 75 FT SET NEW CSG 3 1/2" IN TO AT 77.712 DEVEGASING TO 78.0 FT W/ 350 lb HAMMER FALLING FREE 3.4 IN CAUSING CLEANING OUT W/ THE LONG ROLLER BIT RESULT TO SLOWING.	
	53				30	80.0	
	79	43	SM				
	44						
	80	44					
445.8	80	SET	Gravelly coarse to fine SAND, little silt, very dense, wet, brown		JAR	DRIVE 1 3/4 x 2.0 x 2 1/2" IN STANDARD SPLIT SPOON W/ 140 lb SAFETY HAMMER FALLING FREE 30 IN	
	81	48			31	DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft	
	59		SM				
	40						
	82	40					

SDG FORM 1836-A

470 1000 07-100-000

PROJECT ONONDAGA D.I. 17

HOLE NO. D82-101

DRILLING LOG (Cont Sheet)			SECTION TOP OF HOLE		525.8 ft MSL		Hole No. DB2-101	
PROJECT			INSTALLATION		BUFFALO DISTRICT		DATE	
ONONDAGA DAM							8	
ELEVATION	DEPTH	LOG NO.	CLASSIFICATION OF MATERIALS (Description)	% CORRECTION	LOG OR SAMPLE NO.	REMARKS (Drilling data, water level, depth of weathering, etc., if significant)	LOG NO.	REMARKS
	82	SET	Gravelly coarse to fine SAND, little silt, very dense, wet, brown SM		81.0	3 AUGUST 1961 AM CSG 8 IN. ID DEPTH TO WATER 65.1 ft BOREHOLE ADVANCED W/ CASE 7.5 TUCKERMAN RITUAL SERIAL # 154115 ADVANCE RECORDS BY DRILLING MACHINERY W/ 350 LB HAMMER FALLING FREE 2 IN BENTONITE WASHED PRIOR TO SAMPLING WITH LONG SLEEVE BIT. DRIVE 2.0 REC 0.8 LOSS 1.2		35
	83	33			JAR 32			47
	84	18			85.0	DRIVE 1 3/4 IN 2.0 X 27 1/2 IN STANDARD SPECT SPOON W/ 40 LB SAFETY HAMMER FALLING FREE 30 IN DRIVE 2.0 ft REC 0.7 ft LOSS 1.3 ft		22
	85	17	Gravelly coarse to fine SAND, little silt, very dense, wet, brown SM		JAR 33			52
	86	26			86.0	DRIVE 2.0 ft REC 0.8 ft LOSS 1.2 ft		29
	87	24			JAR 34			44
	88	38	Gravelly coarse to fine SAND, little silt, very dense, wet, brown SM		88.0	DRIVE 2.0 ft REC 0.5 ft LOSS 1.5 ft		34
	89	35			JAR 35			31
	90	30			90.0	DRIVE 2.0 ft REC 0.5 ft LOSS 1.5 ft		28
	91	26	Gravelly coarse to fine SAND, little silt, very dense, wet, brown SM		JAR 36	NOTE: LARGE GRAVEL PIECES OFTEN FOUND LODGED IN SPOON. SAMPLES FROM 78.0 TO 76.0 ft		45
	92	29			92.0	DRIVE 2.0 ft REC 0.5 ft LOSS 1.5 ft		28
	93	27			JAR 37			25
433.8	94	37	silty coarse to fine SAND little gravel, very dense wet, brown SM		94.0			

END FORM 1836-A

DATE: 1961-08-08

PROJECT

ONONDAGA DAM

FILE NO.

DB2-101

DRILLING LOG (Cont Sheet)			ELEVATION TOP OF HOLE		525.84 MSL		Hole No. D82-101	
CHONDAGA DAM			INSTALLATION		BUFFALO DISTRICT		PAGE 9	
REVISION	DEPTH	LOG NO.	CLASSIFICATION OF MATERIALS (Description)		NO. OF RECS. REV.	NO. OF SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)	
	34	SET	Silty coarse to fine SAND, little gravel, very dense, wet, brown SM			94.0	DRIVE 1 3/8 x 2 x 2 1/2" STANDARD SLIST SHOWN W/ 14016 SAFETY HAMMER FALLING FREE 30 IN ADVANCE BOREHOLE W/ AIR CASING DRIVEN W/ 350 LB HAMMER FALLING FREE 24 IN DRIVE 2.0 ft REC 0.3 ft LOSS 1.7 ft	
	35					JAR		
	37					38		
429.8	36					96.0		
	14	SET	Coarse to fine sandy GRAVEL, little silt, dense wet, brown GM			JAR	DRIVE 2.0 ft REC 0.7 ft LOSS 1.3 ft	
	25					39		
	27					98.0		
	22	SET	Coarse to fine sandy GRAVEL, little silt dense, wet, brown GM			JAR	DRIVE 2.0 ft REC 0.8 ft LOSS 1.2 ft	
	26					40		
	24					100.0		
425.8	100					100.0		
	26	SET	Silty coarse to fine SAND, little gravel, dense, wet, brown SM			JAR	DRIVE 2.0 ft REC 0.6 ft LOSS 1.4 ft	
	23					41		
	24					102.0		
423.8	102			102.0		JAR	DRIVE 1.3 ft REC 0.4 ft LOSS 0.9 ft	
	25	SET	Coarse to fine sandy GRAVEL, little silt, very dense, wet, brown GM			42		
	100					103.3		
422.8	100					103.3		
	104		APPROXIMATE TOP OF ROCK 103.3 MASSIVE LIMESTONE, OLNEY MEMBER LIMESTONE, moderately hard, dense to finely crystalline flat lying, few shalloles, slightly weathered, solid, dark gray Frequent fractures in the core are mechanical in nature caused by drilling. PULL #1 RAN 1.9 ft REC 1.6 ft UNACCOUNTABLE LOSS 0.3 ft BOTTOM OF BOREHOLE 106.7 ft			NS	SPONGE REVERSAL 103.3 ft ADVANCE CASING TO 103.2 ft W/ 350 LB HAMMER FALLING FREE REVAL USED RE- CORRE ROLLER SET TO 103.8 ft TO CLEAR PRESUMED OBSTRUCTION. CUTTINGS INDICATED ROCK. ATTEMPTED TO CORE W/ NX DOUBLE THREADED BARREL (3 1/2" ID) BUT WAS UNABLE, DUE TO BENT CASING AT BOTTOM OF QUESTING. ATTEMPTED TO CORE W/ 6 1/4" ID DOUBLE THREADED BARREL (1 1/2" ID) DRILLED 1.9 ft BEFORE CORE BARREL BROKE OFF. DRILLING TIME WAS 6 MIN 19. HOLE WAS TERMINATED AND CORE BARREL RECOVERED AT A LATER DATE. CORE HOLE CEMENTED TO SURFACE TAGS COSTS 2 103.2 ft. MORE 65.2 ft	
	105					103.8		
	106					PULL		
420.4	106					105.7		

SDS FORM 1834-A

DATE: 1980-07-20

CHONDAGA DAM

D82-101

Hole No. DB2-102

DRILLING LOG		LOCATION		PROJECT	
ONONDAGA DAM		NORTH CENTRAL		BUFFALO DISTRICT	
1. PROJECT		2. DATE AND TYPE OF TEST		3. SHEET / 15 SHEETS	
4. LOCATION (Township or Section)		5. DATE OF TEST		6. TESTER	
7. TESTER		8. DATE OF TEST		9. TESTER	
10. HOLE NO. (as shown on drawing and not the number)		11. TOTAL NO. OF TESTS		12. TOTAL NO. OF TESTS	
13. NAME OF DRILLER		14. TOTAL NUMBER CORE SECTIONS		15. ELEVATION OF WATER	
16. DIRECTION OF HOLE		17. DATE HOLE		18. ELEVATION TOP OF HOLE	
19. THICKNESS OF OVERBURDEN		20. ELEVATION TOP OF HOLE		21. TOTAL CORE RECOVERY FOR BORING	
22. DEPTH DRILLED INTO ROCK		23. SIGNATURE OF INSPECTOR		24. SIGNATURE OF INSPECTOR	
25. TOTAL DEPTH OF HOLE		26. SIGNATURE OF INSPECTOR		27. SIGNATURE OF INSPECTOR	

ELEVATION	DEPTH	LEGEND	CLASSIFICATION OF MATERIALS	REMARKS	BOX OR SAMPLE NO.	REMARKS
525.7	0	SET	Coarse to fine sandy GRAVEL, little silt, very dense, dry, brown GM	0.0	1	ADVANCE BORING W/ 1.75 IN ID MASON STEEL AUGER W/ PLUG - MFG CMC
	13				JAR 1	DRIVE 3.0 ft REC 3.0 ft LOSS 0.0 ft
	37					DRIVE 2.0 ft REC 1.5 ft LOSS 0.5 ft
	27			B	2.0	
	35	SET	Coarse to fine sandy GRAVEL, little silt, dense, dry, brown GM	A	JAR 2	DRIVE 1.7 ft REC 1.7 ft LOSS 0.0 ft
	50			G		
	50			1	37	
521.7	4	SET	Silt, coarse to fine SAND, little gravel, very dense, dry, brown SM	4.0	NS	ADVANCE BORING TO 4.0 ft W/ 1.75 IN ID MASON STEEL AUGER W/ PLUG - MFG CMC
	43				JAR 3	DRIVE 1.4 ft REC 1.4 ft LOSS 0.0 ft
	50			5.4	5.4	
520.3	6	SET	Gravelly, coarse to fine SAND, little silt, dense, damp, brown SM	6.0	NS	ADVANCE BORING TO 6.0 ft W/ 1.75 IN ID MASON STEEL AUGER W/ PLUG - MFG CMC
	42			B	JAR 4	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
	36			A		
	32			G	8.0	
	20	SET	Gravelly, coarse to fine SAND, little silt, dense, damp, brown SM		JAR 5	DRIVE 1.9 ft REC 1.9 ft LOSS 0.0 ft
	27					
	50				99	

BOG FORM 1836 PREVIOUS EDITIONS ARE OBSOLETE.
MAR 71 (TRANSPERIT)

PROJECT

ONONDAGA DAM

HOLE NO.

DB2-102

DRILLING LOG (Cont Sheet)			STATION TOP OF HOLE		HOLE NO.	
PROJECT			DISTRICT		SHEET	
ONONDAGA DAM			BUFFALO DISTRICT		OF 5 SHEETS	
ELEVATION	DEPTH	LOGS	CLASSIFICATION OF MATERIALS (Description)	B CODE RECOV. BY	SOIL OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)
	10	SET	Gravelly coarse to fine SAND, little silt, very dense, damp, brown	B A G 2	JAR 6	DRIVE 0.8 ft REC 0.8 ft LOSS 0.0 ft
	11	50	SM	10.8	10.8	
	12	SET	Gravelly coarse to fine SAND, little silt, very dense, damp, brown		NS	ADVANCE BORING TO 11.0 ft w/ 3.75 IN HOLLOW STEM AUGER w/ PLUG MFG-CME DRIVE 3.0 ft 3.5 IN 38 TN SPRINTSOUND w/ 3.5 IN 1/2 IN FRESH FILL 18 TN
	13	37	SM		JAR 7	DRIVE 1.9 ft REC 1.9 ft LOSS 0.0 ft
	14	50		B	42.9	
	14	SET	Gravelly coarse to fine SAND, little silt very dense, damp, brown	A	41.0	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
	15	48	SM	G	JAR 8	6 AUG 1982 CONCRETE STRUCTURE TO CHIEF'S SEAT 4 139795
	16	37		3	16.0	
	16	SET	Gravelly coarse to fine SAND, little silt, very dense, damp, brown		JAR 9	DRIVE 0.4 ft REC 0.4 LOSS 0.0 STEM REFUSAL TO DRIVE 0.4 ft
	17	50	SM		NS	ADVANCE BORING TO 18.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG-CME
	18	SET	Gravelly coarse to fine SAND, little silt, very dense, damp, brown		JAR 10	DRIVE 1.3 ft REC 1.3 ft LOSS 0.0 ft
	19	50	SM		17.3	
	20	SET	Gravelly coarse to fine SAND, little silt, dense, moist, brown		NS	ADVANCE BORING TO 20.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG-CME
	21	12			20.0	DRIVE 2.0 ft REC 1.3 ft LOSS 0.7 ft
	21	21	SM		JAR 11	
503.7	22	34		22.0	22.0	

END FORM 1836-A

NO. 100-100-100

PROJECT ONONDAGA DAM HOLE NO. DB2-102

DRILLING LOG (Cont Sheet)			STATION TOP OF HOLE		HOLE NO.	
PROJECT			INSTALLATION		DATE	
ONONDAGA DAM			BUFFALO DISTRICT		10/5/50	
STATION	DEPTH	LOGGING	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	SOIL OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of penetration, etc., if significant)
503.7	23	SET	Coarse to fine sandy GRAVEL, little silt, dense, damp, brown GM	22.0	22.0	ADVANCE BORING W/ 3/8" IN ID HOLLOW STEM AUGER W/ PLUG MFG-CME DRIVE 30x35x38 JAR SPUR IN W/ 350 lb HAMMER FALLING FREE 19 IN DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
501.7	24	SET	Medium to fine SAND, little silt, trace gravel, medium dense, damp, brown SM	24.0	24.0	DRIVE 1.7 ft REC 1.7 ft LOSS 0.0 ft
501.2	25	21	Gravelly coarse to fine SAND, little silt, very dense, damp, brown SM	25.5	25.5	
500.3	30	60	Medium to fine SAND, little silt very dense, damp, brown SM	25.4	25.4	
499.7	26	SET	Coarse to fine sandy GRAVEL, little silt dense, damp, brown GM	26.0	26.0	ADVANCE BORING TO 26.0 ft W/ 3/8" IN ID HOLLOW STEM AUGER W/ PLUG MFG-CME
499.2	27	31	Medium to fine SAND, trace silt, trace gravel, very dense, damp, brown SP	26.5	26.5	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
	28	SET	Medium to fine SAND trace to 1 1/2% silt, trace gravel, very dense, damp brown SP	28.0	28.0	DRIVE 1.3 ft REC 1.3 ft LOSS 0.0 ft
	30	SET	Medium to fine SAND, trace silt, trace gravel very dense, damp, brown SP	30.8	30.8	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
494.9	31	45	Coarse to fine sandy GRAVEL little silt, very dense, damp brown GM	32.0	32.0	DRIVE 1.3 ft REC 1.3 ft LOSS 0.0 ft
	32	SET	Coarse to fine sandy GRAVEL, little silt, very dense, damp, brown GM	33.3	33.3	ADVANCE BORING TO 34.0 ft W/ 3/8" IN ID HOLLOW STEM AUGER W/ PLUG MFG-CME
491.7	24			34.0	34.0	

ENG FORM 1000-A

USE PREVIOUS EDITIONS

PROJECT

ONONDAGA DAM

HOLE NO.

102-102

DRILLING LOG (Cont Sheet)		REVISION TOP OF HOLE		525.74 MSL		Hole No. 102-102	
PROJECT		LOCATION		DISTRICT		DATE	
ONONDAGA DAM		BUFFALO DISTRICT		10/15/44			
DEPTH	LOGS	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERY	LOG OR SAMPLE NO.	REMARKS (Drilling time, casing loss, depth of penetration, etc., if significant)		
411.7	34	SET	34.0	34.0	DRIVE 1.3 ft REC 1.3 ft LOSS 0.0 ft		
35	56	Coarse to fine SAND, Some gravel, little silt, very dense, damp. brown	35.3	JAR 22			
	50	SM		NS	ADVANCE BORING TO 36.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG. CME		
409.7	36	SET	36.0	36.0	DRIVE 0.8 ft REC 0.8 ft LOSS 0.0 ft		
	50	Coarse to fine sandy GRAVEL, little silt, very dense, damp, brown		JAR 23			
37		GM		NS	ADVANCE BORING TO 38.0 ft w/ HOLLOW STEM AUGER w/ PLUG MFG. CME DEEPEST 1582 ft DEPTH TO BOTTOM OF CSG 38.0 ft		
38	SET	Coarse to fine sandy GRAVEL, little silt, very dense, damp, brown		JAR 24	DRIVE 0.7 ft REC 0.7 ft LOSS 0.0 ft		
	50	GM		NS	ADVANCE BORING TO 40.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG - AIRG CME		
39							
40	SET	Coarse to fine sandy GRAVEL, little silt, very dense, damp, brown		JAR 25	DRIVE 0.5 ft REC 0.3 ft LOSS 0.2 ft		
	50	GM		NS	SPONGE REFUSAL @ 40.5 ft BS DROPS 10.5 ft ADVANCE BORING TO 42.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER - NO PLUG CLEAN REMOVABLE w/ TRI CONE SET TO 42.0 ft AND CONTINUE BORING w/ 3.0 x 3.5 x 3.5 ft SPIT SPONGE DROPS w/ 3.0 x 3.0 IN. HOLLOW STEM 18 ft.		
41							
42	SET	Coarse to fine sandy GRAVEL, little silt, very dense, damp, brown		JAR 26	DRIVE 1.0 ft REC 0.3 ft LOSS 1.7 ft		
	85	GM		NS	ADVANCE BORING TO 44.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG. CME		
43							
44	SET	Coarse to fine sandy GRAVEL, little silt, very dense, wet, brown		JAR 27	NOTE: AUGERING DIFFICULT DUE TO 1.5 IN. 2 IN. AUGERS TO CHECK BIT, REPLACED 1 DOWN. ADVANCED BORING TO 44.0 ft w/ HOLLOW STEM AUGERS w/ PLUG. CLEANED OUT AUGERS w/ TRI CONE ROLLER BIT RESUME BORING @ 44.0 ft		
	41	GM		NS	DRIVE 1.8 ft REC 1.3 ft LOSS 0.5 ft		
45							
46	50			NS	ADVANCE BORING TO 46.0 ft		

ENG FORM 1836-A

ONONDAGA DAM

102-102

DRILLING LOG (Cont Sheet)		ELEVATION TOP OF HOLE		525.7 ft MSL		Hole No. D82-102	
PROJECT		INSTALLATION		SHEET		OF 5 SHEETS	
ONONDAGA DAM		BUFFALO DISTRICT					
ELEVATION	DEPTH	LOGS	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	SOIL OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)	
478.8	46	SET	Coarse to fine sandy GRAVEL, little silt, very dense, wet, brown SM	46.9	46.0	DRIVE 0.9 ft REC 0.5 ft LOSS 0.4 ft	
	47				NS	ADVANCE BORING TO 48.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/PLUG DRILLER NOTES DIFFICULT AUGERING	
	48	SET	Gravelly coarse to fine SAND, little silt, very dense, wet, brown SM		48.0	DRIVE 0.8 ft REC 0.8 ft LOSS 0.0 ft	
	49				NS	ADVANCE BORING TO 50.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/PLUG NAUG 82. MM DEPTH TO 50.0 ft w/ 3.75 IN ID DEPTH TO 50.0 ft w/ 3.75 IN ID	
	50	SET	Gravelly coarse to fine SAND, little silt, very dense, wet, brown SM		50.0	DRIVE 0.9 ft REC 0.9 ft LOSS 0.0 ft	
	51				NS	ADVANCE BORING TO 52.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER AND CONTINUOUS RECORDING w/ 2.5 x 3.5 x 58 SHEET SPANNING DIALS w/ 2.0 x 1.5 HANGER FALLING PREC 18 IN	
	52	50	Gravelly coarse to fine SAND, little silt, very dense, wet, brown SM		52.0	DRIVE 0.0 REC 0.0 LOSS 0.0	
	53				NS	ADVANCE BORING TO 54.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/PLUG. MFG C-100	
	54	SET	Gravelly coarse to fine SAND, little silt, very dense, wet, brown SM		54.0	DRIVE 0.8 ft REC 0.6 ft LOSS 0.2 ft	
	55				NS	ADVANCE BORING TO 56.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/PLUG. MFG C-100	
	56	SET	Gravelly coarse to fine SAND, little silt, very dense, wet, brown SM		56.0	DRIVE 0.6 ft REC 0.4 ft LOSS 0.2 ft	
	57				NS	ADVANCE BORING TO 58.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/PLUG NOTE: AUGERING DIFFICULT	
	58						

SDS FORM 1006-A

REV. 10-1-66

PROJECT

ONONDAGA DAM

HOLE NO.

D82-102

DRILLING LOG (Cont Sheet)		ELEVATION TOP OF HOLE		525.7 ft MSL		Hole No. D82-102	
PROJECT		ONONDAGA DAM		BUFFALO DISTRICT		DATE 8/15/88	
ELEVATION	DEPTH	LOGS	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERY	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if applicable)	CSG PLAS PER FOOT
58	SET		Gravelly coarse to fine sand, little silt, very dense moist, brown SM	B	38.0	DRIVE 0.5 ft REC 0.5 ft LOSS 0.0 ft	
59				A	38.5	ADVANCE BORING TO 40.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG APC - CAF	
60	SET		Gravelly coarse to fine sand, little silt, very dense, moist, brown SM	G	40.0	DRIVE 1.7 ft REC 1.7 ft LOSS 0.0 ft	
61	50			9	JAR 35		
	36						
	50				61.7		
62						AUGER W/ PLUG 1.7 IN USED CHANGE THE CONE AUGER SET FROM 61.7 TO 63.0 ft ADVANCE 3.75 IN ID HOLLOW STEM AUGER'S TO 63.0 ft w/ PLUG. SM of RUNNING SAND IN AUGER'S. UNABLE TO CLEAN OUT AUGER'S w/ THE CONE RYCEA. SET REEL AUGER'S TO 60.0 ft AND SET ANCHORING 3 ft. DRIVE CONE TO 60.0 ft AND RESUME CONE. LOSS SAMPLING w/ 1/2 IN 2.0 ft 27% TO SPLIT SHAFT. THE 6.0 ft LOSS SETTING AUGER. REEL AUGER	35
461.7	64		Coarse to fine sandy GRAVEL, little silt, very dense, wet, brown GM	64.0	64.0	DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft	25
	50				JAR 36		
460.2	44			65.5	65.5		32
	25		Fine sandy SILT & CLAY very stiff, damp, gray CL		JAR 37		
	66				66.0		
	SET					DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	39
	14		Fine sandy SILT & CLAY very stiff, damp, brown CL		JAR 38		
	67						68
	15						
457.7	68			68.0	68.0		
	SET		SILT, little clay trace medium to fine sand, medium dense, wet brown ML		JAR 39	DRIVE 2.0 ft REC 0.4 ft LOSS 1.6 ft	35
	69						
	14						
	12					NOTE: THE POOR RECOVERY MAY BE DUE TO GRAVEL AND/OR COBBLE BEING BURIED AHEAD BY THE SPIN	62
455.7	70			70.0	70.0		62

SDS FORM 1836-A

SPD 1000 07-100-000

PROJECT ONONDAGA DAM

HOLE NO D82-102

DRILLING LOG (Cont Sheet)			ELEVATION TOP OF HOLE		525.7 # MSL		Hole No. DB2-102	
PROJECT			ONONDAGA DAM		SHEET NO.		DATE	
LOCATION			BUFFALO DISTRICT		SHEET		DATE	
ELEVATION	DEPTH	LOG NO.	CLASSIFICATION OF MATERIALS (Description)	% CORN RECOV. BY	BOX OR SAMPLE NO.	REMARKS (Drilling rate, water loss, depth of penetration, etc., if significant)	CSG BLW PER FEET	
455.7	10	SET	Silty fine SAND, trace organics, wood & shell fragments, dense, moist, gray SM		70.0	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	60	
454.2	15				JAR 40		60	
454.0	19				76.5		66	
453.7	71	14	Coarse to fine sand, CLAYEY, little silt, trace organics, stiff, moist, gray SM 71.7	71.5	71.5		66	
	72	14	Clay, SET, little fine sand, trace silt, gray, trace organics, stiff, moist, gray ML 72.0	71.7	71.7		66	
	72	SET			72.0	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft	56	
	73	17			JAR 43	DRILLING ADVANCED WITH WATER 3 3/4 IN. ID DRIVEN W/ 350 LB HAMMER FALLING FREE 2 FEET SAVINGS CONDUCTED W/ 1 3/8" 2.0 x 27 1/2 IN. STANDARD SPITS POORLY DRIVEN W/ 100 LB. (ART) HAMMER W/ 1 3/4 IN. ID	58	
	74	18			74.0		58	
	75	SET	Silty coarse to fine SAND, trace gravel dense, wet, gray SM			DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft	56	
	75	15			JAR 44		56	
	76	25			76.0		49.5	
449.7	76	SET	Gravelly coarse to fine SAND, little silt, dense wet, gray SM			DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft	72	
	77	15			JAR 45		63	
	78	13			78.0		63	
	79	SET	Gravelly coarse to fine SAND, little silt, trace wood fragments, medium dense, wet, gray SM			DRIVE 2.0 ft REC 0.8 ft LOSS 1.2 ft	50	
	79	16			JAR 46		78	
	80	15			80.0		78	
445.7	80	SET	Silty coarse to fine SAND little gravel, very dense, wet, brown SM			DRIVE 2.0 ft REC 0.8 ft LOSS 1.2 ft	75	
	81	31			JAR 47		75	
	81	33					75	
	82	26			82.0		75	

DRILLING LOG (Cont. Sheet)		ELEVATION TOP OF HOLE		525.74 MSL		Hole No. 182-102		DATE		8	
PROJECT		CHONDAGA DAM		DISTRICT		BUFFALO DISTRICT		DATE		15	
ELEVATION	DEPTH	LOGNO	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	LOG NO.	LOG NO.	LOG NO.	LOG NO.	LOG NO.	LOG NO.	LOG NO.
	82	SET	Silty coarse to fine SAND, little gravel, very dense, wet, brown		82.0						
	38				JAR						
	83	30			48						
	24		SM		94.0						
	84	SET	Silty coarse to fine SAND, little gravel, dense, wet, brown								
	20				JAR						
	85	19	SM		49						
	24				86.0						
435.7	86	SET	Gravelly coarse to fine SAND, little silt, dense wet, brown								
	21				JAR						
	87	18	SM		50						
	15				88.0						
	88	SET	Gravelly coarse to fine SAND, little silt, dense, wet, brown		JAR						
437.2					51						
	22		Silty medium to fine SAND, trace gravel, coarse sand, wet, brown		JAR						
436.7	89	26	Gravelly coarse to fine SAND, little silt, very dense, wet, brown		52						
	26		SM		53						
90		SET	Gravelly coarse to fine SAND, little silt, very dense, wet, brown								
	36				JAR						
	89		SM		54						
	28				92.0						
91		SET	Gravelly coarse to fine SAND, little silt, very dense, wet, gray								
	29				JAR						
	89		SM		55						
	19				94.0						
431.7	94										

END FORM 1826-A

CHONDAGA DAM DB2-102

DRILLING LOG (Cont Sheet)			SECTION TOP OF HOLE		Hole No. DB2-102		CSG BLAU PER FEET
PROJECT ONONDAGA DAM			INSTALLATION 25.7 ft MSL BUFFALO DISTRICT		DATE 12/15/87		
ELEVATION	DEPTH	LOGNO	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)	
431.7	94	SET	Silty coarse to fine SAND trace gravel, dense, wet gray SM	94.0	JAR 56	DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft	87
430.7	95	21	85.0	95.0	JAR 57		
430.2		25	Medium to fine SAND, little silt, trace coarse sand, dense, wet, gray SM	95.5	JAR 58		765
	96	25	Silty fine SAND, trace coarse to medium sand, dense, wet, gray SM	96.0	JAR 59	BORE HOLE ADVANCED W/ NW CASING 3 7/16 IN ID W/ 350 LB HAMMER FALLING FREE 2.4 IN SAMPLING CONDUCTED W/ 1 7/8 IN 20% 2 1/2 STANDARD SPLIT SPOON SAMPLER DRIVEN W/ 140 LB SAFETY HAMMER FALLING FREE 30 IN. DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft	54
428.7	97	21	Silty fine SAND, trace coarse to medium sand, dense wet, gray SM	97.0	JAR 60		67
	98	20	Coarse to fine SAND, little gravel, little silt, dense, wet, gray SM	98.0	JAR 61	DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft	57
	99	20	Coarse to fine SAND, little silt, little gravel, dense, wet, gray SM				65
		18	SM				
		15		100.0	100.0		
425.7	100	SET	Medium to fine SAND, little silt, dense, wet, gray SM		JAR 62	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft	81
424.7	101	27		101.0	101.0		
		29	Gravelly coarse to fine SAND, little silt, very dense, wet, gray SM		JAR 63		754
		46	SM	102.3	102.0		
423.4	102	SET	Silty medium to fine SAND, medium dense, wet, gray SM	102.6	JAR 65	DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft	65
423.1	103	17	Coarse to fine SAND, little silt, little gravel, medium dense, wet, gray SM		JAR 66		56
	104	15	SM		104.0		
		SET	Coarse to fine SAND, little silt, little gravel		JAR 67	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft	47
	105	14	SM				
		18					82
	106	25			106.0		

SDO FORM 1836-A

DATE 12/15/87

PROJECT

ONONDAGA DAM

HOLE NO.

DB2-102

DRILLING LOG (Cont. Sheet)		REVISION TOP OF HOLE		Hole No. D82-102		
PROJECT		LOCATION		SHEET		
ONONDAGA DAM		BUFFALO DISTRICT		10		
ELEVATION	DEPTH	LOG NO.	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERY	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)
417.7	106	SET	Coarse to fine SAND, little gravel, little silt, dense, wet, gray SM		106.0	DRIVE 2.0 ft REC 1.3 ft LOSS 0.7 ft
	107	21		JAR	SAMPLING CONDUCTED BY 1 7/8 x 2.0 x 27 1/2 IN STANDARD SPLIT SPIN DRIVEN BY 140 LB SAFETY HAMMER FALLING FREE DOWN	
	26	68				
	28	108.0				
413.7	108	SET	Silty fine SAND, trace coarse to medium sand, trace gravel, dense, wet, gray SM		108.0	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft
	109	18		JAR	110.0	
	22	69				
	31	110.0				
	110	SET	Silty fine SAND, trace gravel, trace coarse to medium sand, dense, wet, gray SM		110.0	DRIVE 2.0 ft REC 1.3 ft LOSS 0.7 ft
	111	19		JAR	112.0	
29	70					
30	112.0					
413.7	112	SET	Coarse to fine sandy GRAVEL, little silt, very dense, wet, gray GM		112.0	13 AUG 82 AM DEPTH TO BOTTOM OF CORE 110.0 DEPTH TO BOTTOM OF HOLE 112.0 DEPTH TO TOP OF WATER 63.4 ft
	113	74		JAR	114.0	
	21	71	DRIVE 2.0 REC 0.5 LOSS 1.5			
	114	20	114.0	DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft		
	115	60	JAR		116.0	
	75	72	ROLLER BIT USED TO CLEAR POSSIBLE OBSTRUCTION. CASING ADVANCED TO 116.0 ft			
	116	75	116.0	DRIVE 0.6 ft REC 0.6 ft LOSS 0.0 ft		
	117	SET	JAR		116.6	
118	73	116.6	USED TIE-CONE ANALYST TO 118.0 ft AND RESUMED SAMPLING.			
			NS			

SDS FORM 1836-A

DATE 07-10-82

PROJECT

ONONDAGA DAM

HOLE NO.

D82-102

DRILLING LOG (Cont Sheet)			ELEVATION TOP OF HOLE 525.7 ft MSL		Hole No. DB2-102	
PROJECT ONONDAGA DAM			DISTANCE 11		SHEET 15	
ELEVATION	DEPTH	LOG NO.	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	BOX OR SAMPLE NO.	REMARKS (Drilling time, water flow, depth of penetration, etc., if significant)
	118	SET			118.0	DRIVE 2.0 ft REC 0.0 ft LOSS 2.0 ft
	119	77			NS	NOTE: COBBLE DRIVEN AHEAD OF SPOON. TRI-CONE ROLLER BIT WAS USED TO ADVANCE CASING TO 120.0 ft.
		88			120.0	KAUG 1982 AM
	120	25	Coarse to fine sandy GRAVEL, little silt, very dense, wet, gray GAI	120.5	JAR 74	DEPTH TO BOTTOM OF CSG 120.0
405.2		SET			120.5	DEPTH TO WATER 85.5 ft
	121	21	Fine sandy SILT, dense, wet, red-brown ML		JAR 75	DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft
		10				
		9				
	122	SET	Fine sandy SILT, trace medium sand, very stiff, damp, red-brown ML	122.5	JAR 76	DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft
403.2		27	Silty fine SAND, very dense wet, red-brown SM		JAR 77	BORING ADVANCED w/ NEW FLUSH JOINT CASING. SAMPLING CONDUCTED w/ 1 1/2 x 20 x 37 1/2 IN STANDARD SPLIT SPOON SAMPLER DRIVEN w/ 140 LB SAFETY HAMMER FALLING FREE 30 IN
	123	36			124.0	
		34				
	124	SET	Silty fine SAND, trace medium sand, w/ occasional silt layers, dense, wet, red-brown SM		JAR 78	DRIVE 2.0 ft REC 1.3 ft LOSS 0.7 ft
	125	22				
		24				
		33		126.0	126.0	
399.7	126	SET	Fine sandy SILT, trace medium sand, dense, wet, red-brown ML	127.0	JAR 79	DRIVE 2.0 ft REC 1.5 ft LOSS 0.5 ft
		34			127.0	
398.7	127	33	Silty fine SAND, very dense, wet, red-brown SM		JAR 80	
		34			128.0	
	128	SET			JAR 81	DRIVE 2.0 ft REC 0.5 ft LOSS 1.5 ft
	129	26	Silty fine SAND, very dense, wet, red-brown SM			
		30			130.0	
395.7	130	27		130.0		

SDG FORM 1836-A

PROJECT ONONDAGA DAM HOLE NO. DB2-102

DRILLING LOG (Cont Sheet)			ELEVATION TOP OF HOLE		Hole No. 262-102	
ONONDAGA DAM			525.7 MSL		SHEET 12 OF 15 SHEETS	
ELEVATION	DEPTH	LEGEND	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOV. BY	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if applicable)
395.7	100	SET	fine sandy SILT, dense wet, red-brown ML		502 82	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
	131	21				
		23				
		25				
393.7	132	SET	Silty fine SAND, to fine SAND, little silt, very dense, wet, red-brown SM		503 83	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft
	133	29				
		34				
		39				
	134	SET	silty fine SAND, very dense, wet, red-brown SM		504 84	DRIVE 2.0 ft REC 1.5 ft LOSS 0.5 ft
	135	27				
		30				
		37				
	136	SET	Silty fine SAND, very dense, wet, red-brown SM		505 85	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft
	137	25				
		27				
		34				
	138	SET	Silty fine SAND, dense wet, red-brown SM		506 86	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
	139	23				
		25				
		30				
	140	SET	Silty fine SAND, dense wet, red-brown with occasional silt layers SM		507 87	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft
	141	15				
		18				
383.7	142	28				

DRILLING LOG (Cont Sheet)			ELEVATION TOP OF HOLE		Hole No.		DATE		SHEET		PAGE	
PROJECT			INSTALLATION		HOLE OR SAMPLE NO.		REMARKS		REMARKS		REMARKS	
ONONDAGA DAM			ROCHESTER DISTRICT		HOLE OR SAMPLE NO.		REMARKS		REMARKS		REMARKS	
ELEVATION	DEPTH	LOGS	CLASSIFICATION OF MATERIALS (Description)	% CORRECTION	RECOVERED	REMARKS	REMARKS	REMARKS	REMARKS	REMARKS	REMARKS	REMARKS
383.7	142	SET	Clayey SILT, stiff, damp red-brown ML	142.0	SAR 88	16 AUG 1962 SET DR CASEING 2.75 IN ID TO 142.0 FT AND RESUME SAMPLING. CASEING DRIVEN BY 350 LB HAMMER FALLING FREE 24 IN. CASEING CLEANED 1 1/4 FEET WITH NET WHICH DETECTS WATER TO THE SIDE. DRIVE 2.0 FT REC 1.0 FT LOSS 1.0 FT						
	24		Fine to medium SAND, little silt, dense, wet, red-brown SM	142.5	SAR 89							
	25											
	23											
	144	SET	Fine to medium SAND, little silt, trace coarse sand, dense, wet, red-brown SM		SAR 90	DRIVE 2.0 FT REC 1.0 FT LOSS 1.0 FT						
	20											
	21											
	19											
	146	SET	Fine to medium SAND, little silt, trace coarse sand, dense, wet, red-brown SM		SAR 91	DRIVE 2.0 FT REC 1.1 FT LOSS 0.9 FT						
	20											
	21											
	24											
	148	SET	Fine to medium SAND, little silt, dense, wet, red-brown SM		SAR 92	DRIVE 2.0 FT REC 1.4 FT LOSS 0.6 FT						
	22											
	23											
	21											
375.7	150	SET	Fine to medium SAND, little silt, trace coarse sand, very dense, wet, gray SM	152.0	SAR 93	DRIVE 2.0 FT REC 1.2 FT LOSS 0.8 FT						
	23											
	30											
	31											
	152											
	153											
371.7	154			154.0	NS	ADVANCE DR CASEING TO 154.0 FT W/ 350 LB HAMMER FALLING FREE 24 IN. RESUME STANDARD SAMPLING AT 5 FT INTERVALS W/ 1 1/2 x 2.0 x 27 1/2 IN STANDARD SLIT TUBING DRIVEN BY 100 LB SAFETY HAMMER FALLING FREE 30 IN						

DRILLING LOG (Cont Sheet)		REVISION TOP OF HOLE		585.7 ft MSL		Hole No. D82-102		
PROJECT		LOCATION		DISTRICT		DATE		
ONONDAGA DAM		BUFFALO DISTRICT		1955		10/25		
REVISION	DEPTH	LOGNO	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	LOG ON SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)		
371.7	154	SET	Silty coarse to fine SAND, little gravel very dense, moist, gray	154.0	154.0	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	156	
	155	53			JAR 94	SAMPLING CONDUCTED BY 1 1/2" x 27" x 2" ID STANDARD SPOON SPOON DRIVEN W/ 140 LB SPOON HAMMER PULSING FREE 30 IN		157
	156	60	SM		156.0	ADVANCE BN CASTING TO 159.0 ft w/ 350 LB HAMMER FREE FALLING 24 IN		158
	157	51			NS			159
	158							160
	159	SET	NOTE: WASH SAMPLE COBBLES & BOULDERS ENCOUNTERED		159.0	DRIVE 0.8 ft REC 0.1 ft LOSS 0.7 ft	161	
	160	100			JAR 95			162
	161				159.8	OBSTRUCTIONS ENCOUNTERED AT 161.1 ft WHILE ADVANCING BN CASTING. OBSTRUCTION COULD NOT BE REMOVED W/ BY PORTALL BIT. REMOVED BN CASTING AND ADVANCED BN CASTING TO 161.1 ft. PRE-CONE ROLLER BIT USED FROM 161.1 TO 162.6 ft THROUGH OBSTRUCTION AND FROM 162.6 TO 164.0 ft THROUGH COBBLES. BN CASTING SET TO 164.0 ft AND SAMPLING RESUMED		163
364.6	161		BOULDER	161.1	NS			164
	162							165
363.1	163			162.6				166
	164	SET	Silty coarse to fine SAND, w/ embedded gravel, very dense, moist, gray		164.0	DRIVE 1.3 ft REC 0.2 ft LOSS 1.1 ft	167	
	165	62	SM		JAR 96	SPOON REFOUR 165.3 ft ROLLER BIT TO 166.8 ft AND ADVANCE CASTING TO 169.0 ft w/ 350 LB HAMMER FALLING FREE RESUME STANDARD SAMPLING AT 169.0 ft		168
	166	100	COBBLES ENCOUNTERED		166.3			169
	167				NS			170

DESLING LOG (Cont Sheet)			REVISION TOP OF HOLE		PROJECT		DATE	
PROJECT			INSTALLATION		REMARKS		DATE	
ELEVATION	DEPTH	LOGNO	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	BOX OR SAMPLE NO.	(Drilling time, water loss, depth of penetration, etc., if applicable)	BLANKS PER FOOT	
	166				JAR 97	UNABLE TO ADVANCE CASING BET 167.7 ft. PENETRATION 167.7 ft. TO 169.0 ft. CORES ENCOUNTERED ADVANCED IN CASING TO 165.0 ft. W/ 350 lb HAMMER. NOTE: SAR 97 IS A WASH SAMPLE FROM 167.7 TO 169.0 ft.	36	
	167						37	
	168						38	
	169	SET			169.0		39	
	170	35	Fine sandy SILT trace clay with embedded coarse to medium sand and gravel, very dense, moist to wet, brown ML		JAR 98	DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft SAMPLES COLLECTED BY 1 1/2 x 2.0 x 27 1/2 IN STANDARD SPLIT SPIN DRIVEN W/ 140 lb SAFETY HAMMER FALLING FREE 30 IN	40	
	171	36			171.0		41	
	172	38				BN CASING ADVANCED TO REFUSAL @ 173.0 ft REMOVED BN CASING TO CHECK POSSIBLE DAMAGE. 0.0 ft OF BN CASING REMAINING IN BOREHOLE FROM 163.0 TO 173.0 ft.	42	
352.7	173		BOTTOM OF BOREHOLE @ 173.0 ft			BOREHOLE TERMINATED AND GRADED TO SURFACE.	43	
	174						44	
	175						45	
	176						46	
	177						47	
	178						48	

Hole No. DB2-103

DRILLING LOG		PROJECT		LOCATION		DATE	
ONONDAGA DAM		NORTH CENTRAL		BUFFALO DISTRICT		1982	
STA 402.5 + 2.4 ON 200 N 5463.02 E H 515.61		CONE 75		ESTIMATED		70	
HOLE NO. 103		DATE HOLE		STARTED		COMPLETED	
184.8 FT		20 AUG 82		27 AUG 82			
ELEVATION TOP OF HOLE		ELEVATION GROUND WATER		ELEVATION TOP OF HOLE		ELEVATION GROUND WATER	
725.8 FT		725.8 FT		725.8 FT		725.8 FT	
THICKNESS OF OVERBURDEN		NOT PENETRATED		NOT PENETRATED		NOT PENETRATED	
DEPTH DRILLED INTO ROCK		NOT ENCOUNTERED		NOT ENCOUNTERED		NOT ENCOUNTERED	
TOTAL DEPTH OF HOLE		184.8 FT		184.8 FT		184.8 FT	
ELEVATION		DEPTH		LEGEND		CLASSIFICATION OF MATERIALS	
525.8		0		SET		Gravelly, coarse to fine SAND, 1.4 ft silt, dense damp, brown SM	
		17					
		25					
		27					
		3					
		4					
520.8		5		SET		Silty coarse to fine SAND, little gravel, dense damp, brown SM	
		34					
519.8		6		50		Gravelly, coarse to fine SAND, 1.4 ft silt, very dense, damp, brown SM	
		7					
		8					
		9					
		10					

ENG FORM 1836
MAR 71PREVIOUS EDITIONS ARE OBSOLETE.
(TRANSLUCENT)

PROJECT

ONONDAGA DAM

HOLE NO.

DB2-103

DRILLING LOG (Cont Sheet)		ELEVATION TOP OF HOLE		INSTALLATION		Hole No.	
PROJECT		525.8 ± MSL		BUFFALO DISTRICT		DB2-103	
ONONDAGA DAM							
ELEVATION	DEPTH	LOGGING	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	SON OF SAMPLE NO.	REMARKS (Drilling tool, water loss, depth of penetration, etc., if significant)	
515.3	10	SET	Gravelly coarse to fine SAND, little silt, dense, damp, brown SM	100	10.0	DRIVE 0.5 ft REC 0.5 ft LOSS 0.0 ft	
	11	50	Silty coarse to fine SAND, little gravel, very dense damp, brown SM	100	10.5		
	12					ADVANCE BORING TO 15.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG. CME SAMPLING CONDUCTED BY 3.0x3.5x38 IN STAND AND SPOT SPIN DRIVEN w/ 375 lb HAMMER FALLING FREE 18 IN.	
513.3	13				12.5		
	14						
	15	SET	Coarse to fine SAND, little gravel, trace to little silt, very dense damp, brown SM	100	15.0	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	
	16	25		B	JAR		
		34		A	6		
	17	33		G	170	ADVANCE BORING TO 20.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG. MFG. CME	
	18				NS		
	19			J			
	20	SET	Coarse to fine SAND, little gravel, little silt, very dense, damp, brown SM		20.0	DRIVE 1.3 ft REC 1.3 ft LOSS 0.0 ft	
	21	49		JAR	7		
		50			21.3	ADVANCE BORING TO 25.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG. MFG CME	
	22			NS			

DRILLING LOG (Cont. Sheet)			SURFICIAL TOP OF ROCK		525.8 ft MSL		Hole No. D82-103	
PROJECT			ONONDAGA DAM		DISTRICT		BUFFALO DISTRICT	
ELEVATION	DEPTH	LOGS	CLASSIFICATION OF MATERIALS (Description)	% CORRECTION	SOIL OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)		
	22				B			
	23				A			
	24				G		NS	
	25				3			
500.2	25	SET	Coarse to fine SAND, little gravel, little silt, very dense, damp, brown SM	25.6	JAR 8	25.0		DRIVE 1.7 ft REC 1.7 ft LOSS 0.0 ft
	26	34	Gravelly coarse to fine SAND, little silt, very dense, damp, brown SM		JAR 9	25.6		
	26	50				26.7		
	27				B			
	28				A			
	29				G		NS	ADVANCE BORE TO 30.0 ft w/ HOLLOW STEM AUGER w/ PLUG MFG CME SAMPLING CONDUCTED BY 3.0 x 3.5 x 3 ft IN STANDARD SPLIT STATION DRIVEN BY 35 lb HAMMER FALLING FREE 7 ft IN
	30	SET	Gravelly coarse to fine SAND, little silt, damp, very dense, brown SM		JAR 10	30.0		DRIVE 1.0 ft REC 1.0 ft LOSS 0.0 ft
	31	55				31.0		
	32							ADVANCE BORE TO 35.0 ft w/ HOLLOW STEM AUGER w/ PLUG MFG-CME
	33						NS	

DRILLING LOG (Cont Sheet)			ELEVATION TOP OF HOLE		HOLE NO.	
PROJECT			HORIZONTAL		DEPTH	
ONONDAGA DAM			BUFFALO DISTRICT		4	
ELEVATION	DEPTH	LOGS	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	SOIL OR SAMPLE NO.	REMARKS (Drilling time, water flow, depth of penetration, etc., if significant)
a	b	c	d	e	f	g
39						
35	3.0					ATTEMPTED TO SAMPLE E 35.0 ft. FROM REFUSAL @ 35.1 ft DRIVE 0.1 ft REC 0.0 ft LOSS 0.1 ft ADVANCE BORING TO 40.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG-CME SAMPLING CONDUCTED BY 3.0X3.5X38 IN SPLIT SPIN DRIVEN w/ 350 LB HAMMER FALLING FREE 18 IN
36						
37					NS	
38						
39						
40				40.0	40.0	
	SET		Gravelly coarse to fine SAND, little silt very dense, damp, brown SM			
41	27			B	JAR	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
	48			A	11	
42	50			G	42.0	
						ADVANCE BORING TO 45.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG-CME
43				5		
44					NS	
45						
	SET		Gravelly coarse to fine SAND, little silt, dense moist, brown SM			
46	18				45.0	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
				JAR	12	

SDS FORM 1836-A

DATE: 1000 07-10-2000

PROJECT

ONONDAGA DAM

HOLE NO.

DBL-103

DRILLING LOG (Cont Sheet)			REMARKS TOP OF HOLE		525.8 ft 115L		Hole No. DB2-103	
PROJECT ONONDAGA DAM			BUFFALO DISTRICT		SHEET 5 OF 16 SHEETS			
DEPTH	LOG NO.	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERY	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of underboring, etc., if significant)			
46	24		B	JAR				
	32		A	12				
47			G					
			5	470				
48					ADVANCE BOREING TO 50.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG-CME			
49					SAMPLING CONDUCTED BY 3.0 x 3.5 x 38 IN SPLIT SPIN DRIVE w/ 36 X HAMMER FALLING FREE BIN.			
50	SET	Silt, coarse to fine SAND, GRAVEL, very dense, damp to moist, brown SM-GM	50.0	50.0				
51	35		B	JAR	DRIVE 2.0 ft			
	50		A	13	REC 1.7 ft			
	47		G		LOSS 0.3 ft			
52			6					
			52.0	52.0				
53					ADVANCE BOREING TO 55.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG-CME			
54					NS			
55	SET	Gravelly coarse to fine SAND, little silt, dense, damp, brown SM	55.0	55.0				
56	16		B	JAR	DRIVE 2.0 ft			
	23		A	14	REC 1.8 ft			
	13		G		LOSS 0.2 ft			
57			7					
			57.0					
58					ADVANCE BOREING TO 60.0 ft w/ 3.75 IN ID HOLLOW STEM AUGER w/ PLUG MFG-CME			
					NS			

DRILLING LOG (Cont Sheet)		ELEVATION TOP OF HOLE		525.8 F MSL		Hole No. DB2-103	
PROJECT		ONONDAGA DAM		METEOROLOGICAL		DATE 8/16/82	
ELEVATION	DEPTH	LOGNO	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOV. BY	SON OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)	
a	b	c	d	e	f	g	
	58						
	59				B	NS	
	60				A		
	60				G	60.0	
	61	23	Gravelly coarse to fine SAND, trace silt, damp, very dense, brown	7	JAR	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	
	61	33	SiM	15		SAMPLING CONDUCTED BY 3.0x3.5x38 IN SPLIT SPOON DRIVEN W/ 350 LB HAMMER FALLING FREE 18 IN.	
	62	32		62.0	62.0		
	63					SET NEW HUSH JARUT CASING 3 3/8 IN ID W 60.0 F. DRIVE TO 65.0 F. WATER LEVEL CHECKED AFTER SETTING CASING BOREHOLE DRY 20 AUG 1982 PM	
	64				NS		
460.8	65			65.0			
460.3	65	SET	Silt, coarse to fine SAND, trace gravel, dense, damp, brown			DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft	
	66	8			JAR	COMMENCE CONTINUOUS SAMPLING USING 1 3/4x2.0x27 IN STANDARD SPLIT SPOON SAMPLER DRIVEN W/ 140 LB SAFETY HAMMER FALLING FREE 30 IN.	
	66	12			16		
	67	18	Silty CLAY, very stiff, damp, gray		JAR	DRIVE 2.0 ft REC 1.5 ft LOSS 0.5 ft	
	67	SET	CL		17		
457.8	68	8		68.0	JAR		
	68	14	SILT, little fine sand, little clay, trace organics (wood fragments)		JAR		
	68	18	dense, damp, gray		18		
	69	SET	SiM		69.0		
	69	6	SILT, little fine sand, little clay, trace gravel, trace wood fragments, medium dense, damp, gray		JAR	DRIVE 2.0 ft REC 1.7 ft LOSS 0.3 ft	
	70				19		

ENG FORM 1836-A
JAN 82

PROJECT ONONDAGA DAM

HOLE NO. DB2-102

DRILLING LOG (Cont Sheet)			ELEVATION TOP OF HOLE		Hole No.		REMARKS (Drilling date, water level, depth of underlying str., if applicable)	SC PCU REL FEET
PROJECT			INSTALLATION		DATE			
ONONDAGA DAM			BUFFALO DISTRICT		7 16			
ELEVATION 1	DEPTH 2	LOGS 3	CLASSIFICATION OF MATERIALS (Description) 4		% CORE RECOVERED 5	SOIL OR SAMPLE NO. 6		
455.3	70	12	Silt, like fine sand, black clay, fine gravel, trace wood fragments and dense damp, gray M 70.5			JAR 19	DRIVE 2.0 ft REC 1.7 ft LOSS 0.3 ft	70
	71	17	Silty coarse to fine SAND, trace gravel SM			JAR 20		
		SET						
	72	18	Silty coarse to fine SAND, little gravel, very dense, damp, gray SM			JAR 21	DRIVE 2.0 ft REC 0.3 ft LOSS 1.7 ft	70.5
		25						71
		32			75.0			
	73	32						
		SET						
	74	38	Silty coarse to fine SAND, little gravel, very dense, dry, gray SM			JAR 22	DRIVE 2.0 ft REC 0.1 ft LOSS 1.9 ft BRING ADVANCED BY 1/4 IN CASING 3 1/2 IN TO DRIVEN W/ 30 LB HAMMER FALLING FREE 24 IN. SAMPLING CONDUCTED W/ 1 1/2 IN 2.0 FT IN STANDARD SPLIT SADDLE DRIVEN BY 140 LB SAFETY HAMMER FALLING FREE 30 IN	75
		32						75.5
448.8	75	30			76.0			76
		SET						
	76	27	Silty coarse to fine SAND, little gravel, very dense damp to wet, gray SM			JAR 23	DRIVE 2.0 ft REC 0.9 ft LOSS 1.1 ft	76
		31						76.5
	77	27			77.0			77
		SET						
	78	27	Gravelly coarse to fine SAND, little silt, very dense, wet, gray SM			JAR 24	DRIVE 2.0 ft REC 0.7 ft LOSS 1.3 ft	77
		31						77.5
		34			77.0			78
		SET						
446.8	80	19	Coarse to fine sandy GRAVEL, little silt, very dense, damp, brown G.K.			JAR 25	DRIVE 2.0 ft REC 0.6 ft LOSS 1.4 ft	78
		30						78.5
		41			81.0			79
444.8	81	41			81.0			79.5
		SET						
82	21		Gravelly coarse to fine SAND, little silt, very dense, moist, wet, brown SM			JAR 26	DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft	80

SDS FORM 1836-A

REV. 1968-01-10

PROJECT ONONDAGA DAM HOLE NO. D82-103

DRILLING LOG (Cont Sheet)		ELEVATION TOP OF HOLE		Hole No.		
PROJECT		INSTALLATION		DATE		
ONDONAGA DAM		BUFFALO DISTRICT		06/16/88		
ELEVATION	DEPTH	LOGGED	CLASSIFICATION OF MATERIALS (Description)	% CORN RECOV. EST	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if applicable)
	82	25	Gravelly coarse to fine SAND, little silt, very dense moist, to wet, brown SM		JAR 26	DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft
	83	35			83.0	
		SET				
	84	52	Gravelly coarse to fine SAND, little silt, very dense moist, brown SM		JAR 27	DRIVE 2.0 ft REC 0.1 ft LOSS 1.9 ft BORING ADVANCED BY NEW CASING 3 1/8 IN ID DRIVEN W/ 350 LB HAMMER FALLING FREE 24 IN. SAMPLING CONDUCTED W/ 1 1/8 x 2.0 x 27 IN STANDARD SPLIT STOOD. DRIVEN BY 140 LB SAFETY HAMMER FALLING FREE 30 IN.
		43			85.0	
	85	30				
		SET				
	86	32	Gravelly coarse to fine SAND, little silt, very dense moist-wet, gray SM		JAR 28	DRIVE 2.0 ft REC 0.7 ft LOSS 1.3 ft
435.3		28			86.5	
438.8	87	25	Silty fine SAND, trace gravel, fine coarse med sand, clean wet red-brown SM 87.0		JAR 29	
		SET			87.0	
	88	27	Silty medium to fine SAND, very dense, wet, red-brown SM		JAR 30	DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft
		24			89.0	
	89	23				
		SET				
	90	19	Silty medium to fine SAND, dense, wet red-brown SM		JAR 31	DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft
		24			91.0	
	91	30				
		SET				
	92	25	Silty medium to fine SAND, very dense, wet, red-brown SM		JAR 32	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft
		23			92.0	
	93	24				
		SET				
	94	23	Silty medium to fine SAND, very dense, wet red-brown SM		JAR 33	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft

SDS FORM 1336-A

PROJECT ONDONAGA DAM HOLE NO. D62-103

DRILLING LOG (Cont. Sheet)			ELEVATION TOP OF HOLE 525.8' NSL		Hole No. D82-103	
PROJECT ONONDAGA DAM			BUFFALO DISTRICT		DATE 10/16/77	
ELEVATION	DEPTH	LOGNO	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	LOG OR SAMPLE NO.	REMARKS (Drilling notes, water level, depth of weathering, etc., if significant)
94	23		Silty medium to fine SAND, very dense, wet, red-brown SM		JAR 32	DRIVE 2.0 ft
					JAR 33	REC 1.4 ft
						LOSS 0.6 ft
95	37		Silty medium to fine SAND, dense, wet, red-brown SM		JAR 34	DRIVE 2.0 ft
						REC 1.4 ft
						LOSS 0.6 ft
96	22				JAR 34	BORING ADVANCED BY NEW CASING 3 1/2 IN ID DRIVEN BY 350 lb HAMMER FALLING FREE 24 IN, SAMPLING CONDUCTED w/ 1 7/8 IN ID STEEL STANDARD SPT (SPT) DRIVEN BY 140 lb SAFETY HAMMER FALLING FREE 20 IN
	27				JAR 34	
97	32		Silty medium to fine SAND, wet, very dense, red-brown SM		JAR 35	DRIVE 2.0 ft
						REC 1.3 ft
						LOSS 0.7 ft
98	35				JAR 35	
	41		NOTE: clayey SPT layer from 98.5 to 98.8 ft		JAR 36	DRIVE 2.0 ft
426.8	99		Coarse to fine SAND, little silt, wet, dense, red-brown SM		JAR 36	REC 1.2 ft
						LOSS 0.8 ft
	25				JAR 37	DRIVE 2.0 ft
						REC 1.3 ft
						LOSS 0.7 ft
424.3	101		Coarse to fine SAND, little silt, trace fine gravel, wet, dense, red-brown SM		JAR 38	DRIVE 2.0 ft
						REC 1.3 ft
						LOSS 0.7 ft
	17		SILT, little fine sand, hard, wet, red-brown		JAR 39	DRIVE 2.0 ft
						REC 1.2 ft
						LOSS 0.8 ft
	19		ML		JAR 40	DRIVE 1.5 ft
						REC 1.2 ft
						LOSS 0.3 ft
422.8	103		Silty fine SAND, w/ occasional clay, SPT bearing, wet, very dense, red-brown SM		JAR 41	DRIVE 1.5 ft
						REC 1.2 ft
						LOSS 0.3 ft
421.8	104		SILT, trace gravel, trace coarse to fine sand, trace clay, hard, damp, red-brown ML		JAR 41	DRIVE 1.5 ft
						REC 1.2 ft
						LOSS 0.3 ft
420.3	106		Silty CLAY, trace sand, trace gravel, trace coarse to fine sand, hard, damp, red-brown CL		JAR 41	DRIVE 1.5 ft
						REC 1.2 ft
						LOSS 0.3 ft

DRILLING LOG (Cont. Sheet)			REVISION TOP OF HOLE		Hole No. DB2-103		CSG BLANK PER FOOT
PROJECT ONONDAGA DAM			BORERATION BOFFALO DISTRICT		SHEET 016 OF 016		
ELEVATION a	DEPTH b	LEGEND c	CLASSIFICATION OF MATERIALS (Description) d	% CORE RECOV. e	SIZE OF SAMPLE NO. f	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant) g	
106	36		Silty CLAY, trace coarse to fine sand, hard, damp, red-brown CL		JAR 41	DRIVE 1.5 ft REC 1.2 ft LOSS 0.3 ft	132
107	54	SET	Silty CLAY, trace coarse to fine sand, hard, damp, red-brown CL		JAR 42	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft BORING ADVANCED BY ANV CASING 3 3/16 IN. DRIVE W/ 350 LB HAMMER FALLING FREE 24 IN. SAMPLING CONDUCTED 4 1/8 x 2.75 IN. STANDARD SPIN. SPIN DRIVEN BY 140 LB SAFETY HAMMER FALLING FREE 30 IN.	110
108	35				JAR 43	DRIVE 1.3 ft REC 1.1 ft LOSS 0.2 ft	112
109	50	SET	Silty CLAY, trace coarse to fine sand, hard, damp, red-brown CL		110.3	USE TRECCO ROLLER BIT + CLEAR POSSIBLE OBSTRUCTION TO A DEPTH OF 111.0 ft. ADVANCE ANV CASING TO 111.0 ft. AND RESUME CONTINUOUS SAMPLING	132
110	50				111.0		
111	25	SET	Silty CLAY, trace coarse to fine sand, hard, damp, red-brown CL		JAR 44	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	135
112	35				113.0		134
113	43	SET	Silty CLAY, trace coarse to fine sand, hard, damp, red-brown CL		JAR 45	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	135
114	35				115.0		131
115	54	SET	Silty CLAY, trace coarse to fine sand, hard, damp, red-brown CL		JAR 46	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft SET AN CASING THROUGH ANV CASING TO A DEPTH OF 115.0 ft. DRIVE CASING TO 117.0 ft. W/ 350 LB HAMMER FALLING FREE 24 IN. AND RESUME CONTINUOUS SAMPLING	123
116	37				117.0		147
117	75	SET	Silty CLAY, trace coarse to fine sand, hard, damp, red-brown CL		JAR 47	DRIVE 2.0 ft REC 1.9 ft LOSS 0.1 ft	139
118	59		Silty fine sand, very dense wet, red-brown SM		JAR 48		

408.5

DB2-103

ONONDAGA DAM

1826-A

1826-A

DB2-103

DRILLING LOG (Cont Sheet)			ELEVATION TOP OF HOLE		Hole No. D82-103	
PROJECT ONONDAGA DAM			525.8 ft MSL		SHEET 11	
INSTALLATION BUFFALO DISTRICT					OF 16 SHEETS	
ELEVATION	DEPTH	LOGEND	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOV. ERY	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)
406.8	118	69	Silty fine SAND, very dense, wet, red-brown		JAR 48	DRIVE 2.0 ft REC 1.9 ft LOSS 0.1 ft
		115	SM	119.0		
	117	SET	SILT, little fine sand occasional clayey		JAR 49	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
		54	SILT seams, hard, wet, red-brown			BRING ADVANCED BY BW CASING 2.75 IN ID DRIVEN W/ 350 LB HAMMER FALLING FREE 24 IN. SAMPLING CONDUCTED W/ 1 1/8 X 1.0 X 27 IN STANDARD SPLIT SPOON DRIVEN BY 140 LB SAFETY HAMMER FALLING FREE 20 IN
	120	50				
		78	ML			
	121	SET	SILT, little fine sand, trace clay, occasional clayey SILT seams,		JAR 50	25 AUG 1982 BOTTOM OF CASING 119.0 ft BOTTOM OF HOLE 121.0 ft DEPTH TO WATER 66.6 ft
		31	hard, damp to wet, red-brown			DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft
	122	36				
		51	ML	123.0		
400.8	123	SET	SILT, little fine sand, trace clay, with clayey SILT, seams and layers, hard, damp to wet, red-brown		JAR 51	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft
		22				
	124	39				
		49	ML	125.0		
	125	SET	Silty fine SAND, trace medium to coarse sand, w/ occasional clayey SILT seams, very dense, wet red-brown		JAR 52	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft
		39				
	126	43				
		54	SM	127.0		
	127	SET	Clayey SILT, little fine sand (seams), hard moist, red-brown		JAR 53	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft
		20				
378.8	128	22	ML			
		25		129.0		
	129	SET	Clayey SILT, little fine sand, (seams) with occasional layers of silty CLAY, red dense, red-brown		JAR 54	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft
130	18					

ENG FORM 1836-A
JAN 67

PROJECT ONONDAGA DAM HOLE NO D82-103

DRILLING LOG (Cont. Sheet)		ELEVATION TOP OF HOLE		525.8 ft MSL		Hole No. D82-103	
PROJECT		ONONDAGA DAM		DISTRICT		BUFFALO DISTRICT	
ELEVATION	DEPTH	LOGGING	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)	FEET
354.8	130	22	Chamber SILT, little fine sand (medium) w/ occasional layers of silty CLAY, very dense, damp, brown		JAR 54	DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	242
	131	35					
	132	SET	Silty fine SAND, very dense, wet, red-brown SM		JAR 55	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft DUE TO HIGH CASING BLOWS FISHTAIL BIT W/ SIDEPORTS UTILIZED FROM 131.0 TO 133.0 ft. CASING ADVANCED TO 133.0 ft. WASHED CUT AND CONTINUOUS SAMPLING RESUMED.	242
	133	37					
	134	51					
	135	83					
	136	SET	Silty fine SAND, very dense, wet, red-brown SM		JAR 56	DRIVE 2.0 ft REC 1.5 ft LOSS 0.5 ft BORING ADVANCED BY BLW CASING 2.75 IN. TO DRIVE W/ 360 lb. HAMMER. FALLING FREE 24 IN. SAMPLING CONDUCTED W/ 1 1/2 x 2.0 x 27 IN. STANDARD CPT. SPIN DRIVEN BY M10 B SAFETY HAMMER FALLING FREE 2 IN.	249
	137	32					
	138	39					
	139	51					
	140	SET	Silty fine SAND, w/ occasional SILT seams very dense, wet, red-brown SM		JAR 57	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft	244
	141	29					
	142	39					
	143	55					
	144	SET	Silty fine SAND, very dense, wet, red-brown SM		JAR 58	DRIVE 2.0 ft REC 1.4 ft LOSS 0.6 ft UTILIZE FISHTAIL BIT W/ SIDEPORTS FROM 137.0 TO 138.7 ft. ADVANCE BLW CASING TO 139.0 ft AND RESUME CONTINUOUS SAMPLING.	248
	145	33					
	146	51					
	147	57					
	148	SET	Silty fine SAND, very dense, wet, red- brown SM		JAR 59	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft UTILIZE FISHTAIL W/ SIDEPORTS FROM 139.0 TO 140.7 ft. ADVANCE CASING TO 141.0 ft WASHOUT AND RESUME CONTINUOUS SAMPLING.	248
	149	30					
	150	48					
	151	62					
	152	SET	Silty fine SAND, very dense, wet, red-brown SM		JAR 60	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft	249
	153	33					

ENG FORM 1836-A

NOV 67

PROJECT ONONDAGA DAM

HOLE NO. D82-103

BELLING LOG (Cont Sheet)			ELEVATION TOP OF HOLE	DATE	WELL NO.	WELL DEPTH
PROJECT			LOCATION	DATE	WELL NO.	WELL DEPTH
ONONDAGA DAM			525.8 ft MSL	1982	103	173
CLASSIFICATION OF MATERIALS (Description)			% CORRECTION	BOX OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of penetration, etc., if significant)	
ELEVATION	DEPTH	LOG NO.				
380.8	142	39		JAR 60	DRIVE 2.0 ft REC 1.6 ft LOSS 0.4 ft	
	143	43		143.0		
	143	SET		JAR 61	DRIVE 2.0 ft REC 1.9 ft LOSS 0.1 ft BORING ADVANCED BY BW CASING 2.75 IN TO DRIVE W/ 350 LB HAMMER FALLING FREE 24 IN. SAMPLING CONDUCTED W/ 1 7/8 x 2.0 IN STANDARD SPLIT SPOON DRIVEN BY M-100 & SAFETY HAMMER FALLING FREE 30 IN	
	144	33		145.0		
	144	37				
	145	50		145.0		
	145	SET		JAR 62	14 AUG 82 AM CSG 6 1430 BOTTOM OF HOLE 145.0 ft DEPT TO HOLE 70.3 ft DRIVE 2.0 ft REC 2.0 ft LOSS 0.0 ft	
	146	21		147.0		
	146	27				
	147	57				
	148			NS	UTILIZED FEMTAL BIT FROM 145.0 TO 149.5 ft ADVANCE CASING TO 150.0 ft. COMMENCE SAMPLING AT 5 ft INTERVALS USING 1 7/8 x 2.0 x 27 IN STANDARD SPLIT SPOON DRIVEN W/ 140 LB SAFETY HAMMER FREE FALLING 30 IN	
	149			150.0		
	150			JAR 63	DRIVE 0.2 REC 0.2 LOSS 0.0	
	151			150.1	UTILIZED BENTONITE AND IN LIEU OF BW CASING FROM 150.0 ft TO BOTTOM OF BORING. WESTERN BRAND BENTONITE, MFG BY BARZO	
	152			NS		
	153					
	154					
376.8	149			149.0		
	150			150.0		
	151			151.0		
	152			152.0		
	153			153.0		
	154			154.0		
372.8	154			154.0		

800 FORM 1836-A

800 FORM 1836-A

PROJECT ONONDAGA DAM WELL NO. 103

DRILLING LOG (Cont. Sheet)			ELEVATION TOP OF HOLE		Hole No.	
PROJECT			INSTALLATION		DATE	
ONONDAGA DAM			BUFFALO DISTRICT		1916	
ELEVATION	DEPTH	LOGNO	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOV. BY	LOG OR SAMPLE NO.	REMARKS (Drilling data, water level, depth of weathering, etc., if significant)
371.8	154	SET	Silty fine SAND, very dense, wet, red-brown SM	154.0	154.0	DRIVE 2.0 ft REC 1.5 ft LOSS 0.5 ft
	155	32			JAR 64	
		38				
	156	47			156.0	
	157				NS	ADVANCE BOREHOLE TO 159.0 ft w/ FISHTAIL BIT. AND DRILLING MUD. RESUME SAMPLING
	158					
	159	SET	Silty fine SAND, very dense, wet, red-brown SM	159.0	159.0	DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft
	160	27			JAR 65	
		38				
	161	47			161.0	
	162				NS	ADVANCE BOREHOLE TO 164.5 ft w/ FISHTAIL BIT. AND DRILLING MUD. RESUME SAMPLING
	163					
	164					
	165	SET	Silty fine SAND, very dense, wet, red-brown SM	164.5	164.5	DRIVE 2.0 ft REC 1.5 ft LOSS 0.5 ft
		22			JAR 66	
	166	32				

DSG FORM 1836-A

DATE 1916

PROJECT ONONDAGA DAM

DATE 1916

Borehole ID# (Cont Sheet)		Borehole ID# of Hole		525.8 ft MSL		Hole No. D82-103	
PROJECT		ONONDAGA DAM		DISTRICT		SHEET 15	
DEPTH	LOGS	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	SOE OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of weathering, etc., if significant)		
166	37	Silty fine SAND, very dense, wet, red brown SM		JAR 66 166.5	DRIVE 2.0 ft REC 1.5 ft LOSS 0.5 ft		
167					ADVANCE BORING TO 167.5 ft w/ FISHTAIL AND DRILLING MUD. RESUME THE SAMPLING.		
168				NS	DRILLER NOTES GRAVEL AND/OR COBBLES ENCOUNTERED BY FISHTAIL BIT FROM 167.5 TO 168.5 ft		
169							
170	SET 19	Silty fine SAND, dense, wet, red-brown SM		JAR 67	DRIVE 2.0 ft REC 1.1 ft LOSS 0.9 ft		
171	26						
172	38			171.5	ADVANCE BORING TO 171.0 ft w/ FISHTAIL BIT AND DRILLING MUD. RESUME SAMPLING.		
173				NS			
174	SET 24	Silty fine SAND, occasional silt, seems very dense, wet, red-brown SM		JAR 68	DRIVE 2.0 ft REC 1.2 ft LOSS 0.8 ft		
175	38						
176	42			176.0	ADVANCE BORING TO 175.0 ft w/ FISHTAIL BIT AND DRILLING MUD. RESUME SAMPLING.		
177				NS			
178							

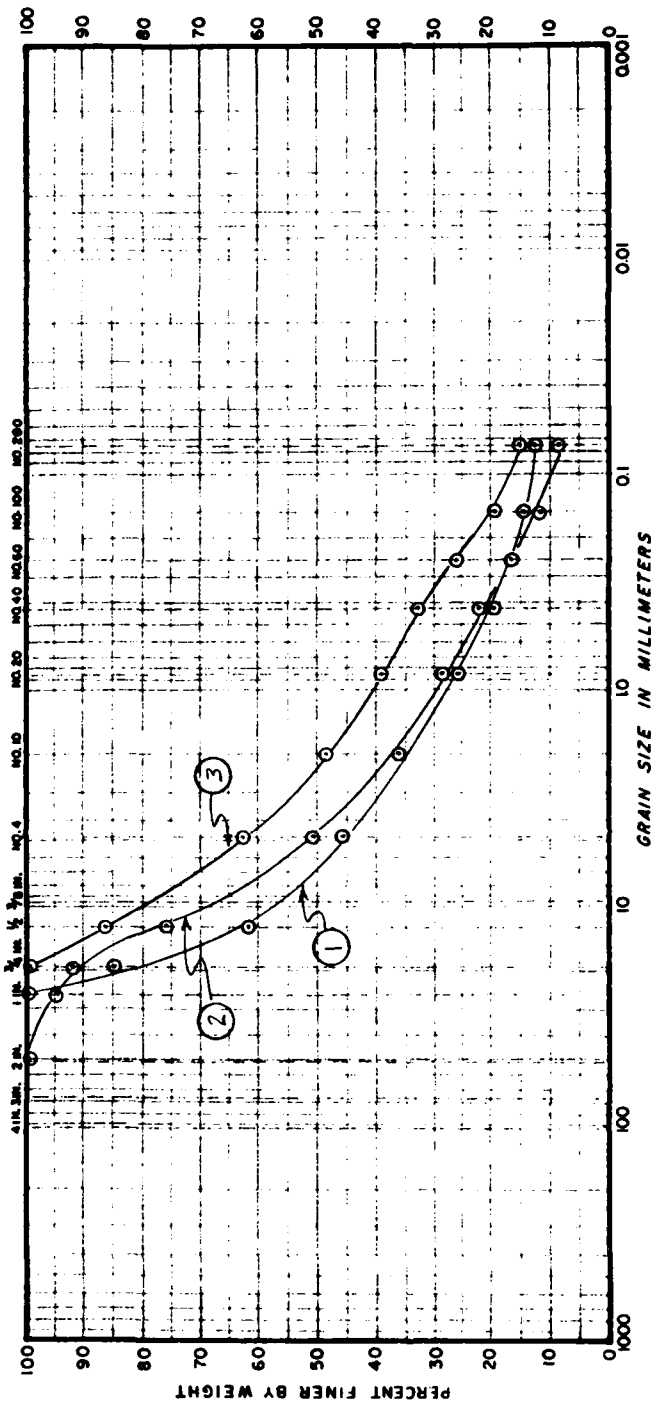
PROJECT		REVISION TOP OF HOLE		Hole No.	
ONONDAGA Dam		SAS. P. 103		102-103	
LOCATION		DISTRICT		DATE	
ONONDAGA		BUFFALO DISTRICT		10/15/68	
DEPTH	LOG NO.	CLASSIFICATION OF MATERIALS (Description)	% CORE RECOVERED	SON OR SAMPLE NO.	REMARKS (Drilling time, water loss, depth of penetration, etc., if significant)
178					ADVANCE BOREHOLE W/ PNEUMATIC BIT TO 179.0 FT. RESUME SAMPLING. DRILLER NOTES GRAVEL AND/OR COBBLES 178.0-178.2 FT
179	SET	Silt, fine sand, very dense, wet, red-brown SM		179.0	DRIVE 2.0 ft REC 1.7 ft LOSS 0.3 ft
180	29			JAR 69	
180	36				
181.8	46		181.0	181.0	ADVANCE BOREHOLE TO 184.0 FT W/ PNEUMATIC BIT AND DRILLING MUD. RESUME SAMPLING. DRILLER NOTES GRAVEL AND/OR COBBLES FROM 181.0 FT TO 184.0 FT
182				NS	
183					
184	SET	Gravelly coarse to fine sand, little silt, very dense, moist, gray SM		184.0	DRIVE 0.8 ft REC 0.3 ft LOSS 0.5 ft
184.8	100			JAR 70	
185		BOTTOM OF BOREHOLE @ 184.8 FT		184.8	BOREHOLE TERMINATED. AND GRADED TO SURFACE.
186					
187					
188					
189					
190					

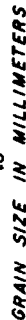


SUMMARY OF LABORATORY TEST RESULTS											
Onondaga Dam Seismic and Geologic Investigations											
BORING & SAMPLE NUMBER	DESCRIPTION	DEPTH (FEET)	TEST NO.	NATURAL WATER CONTENT %	ATTERBERG LIMITS %		UNIT WEIGHT LB/CU FT	UNCONFINED TEST		CONSOLIDATION	
					LL	PL		COMPRESSIVE STRENGTH PSF	STRAIN %	MAX. PAST PRESSURE TON/SQ. FT.	OTHER TESTS
D82-101/ S17	Brown, fine sandy SILT and CLAY, trace fine gravel, trace coarse to medium sand. -CL-	60.0- 62.0	A-1	19.6	23.4	13.6					Hydrometer H-1
D82-101/ S20	Brown, silty CLAY, little fine sand. -CL-	64.0- 65.5	A-2	19.3	38.2	18.1					Hydrometer H-2
D82-102/ S38	Brown, fine sandy SILT and CLAY. -CL-	66.0- 68.0	A-3	20.6	27.4	17.0					Hydrometer H-3
D82-103/ S19	Gray, fine sandy SILT and CLAY, trace coarse to medium sand. -ML-CL-	69.0- 71.5	A-4	23.1	22.1	16.0					Hydrometer H-4
D82-103/ S44	Red-brown, silty CLAY trace coarse to fine sand. -CL-	110.0- 113.0	A-5	13.6	21.7	12.9					Hydrometer H-5

GRAIN SIZE DISTRIBUTION

U. S. STANDARD SIEVE SIZE



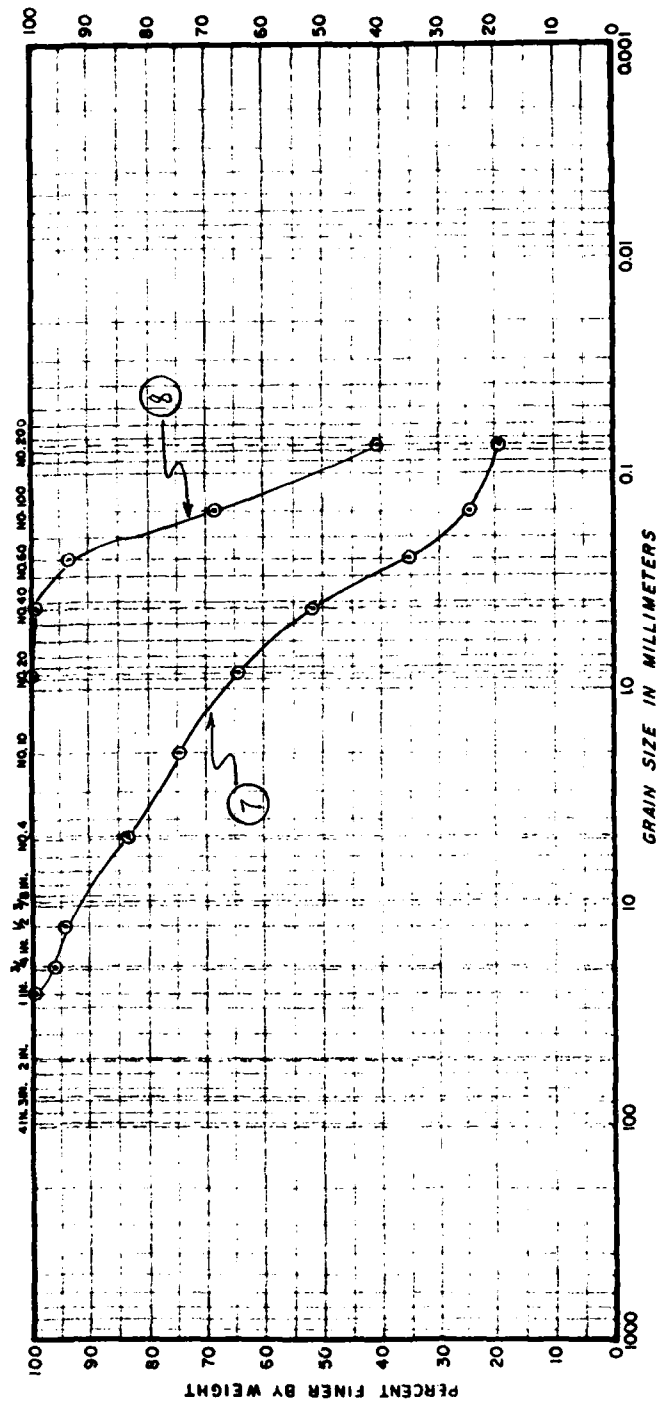
UNIFIED SOIL CLASSIFICATION SYSTEM, CORPS OF ENGINEERS, U.S. ARMY

Test No.	Boring/Sample	Depth (ft.)	Description	PROJECT	FILE NO	DATE
4	D82-102/S12	22.0-24.0	Brown, coarse to fine sandy GRAVEL, little silt. -GM-	Onondaga Dam	745003	August 1982
5	D82-102/S27	44.0-45.8	Brown, coarse to fine sandy GRAVEL, little silt. -GM-	Seismic & Geologic Investigations		
6	D82-102/S46	78.0-80.0	Brown, gravelly coarse to fine SAND, little silt. -GM-			

HALEY & ALDRICH, INC.

GRAIN SIZE DISTRIBUTION

U.S. STANDARD SIEVE SIZE



COBBLES	GRAVEL		SAND			SILT OR CLAY
	COARSE	FINE	COARSE	MEDIUM	FINE	

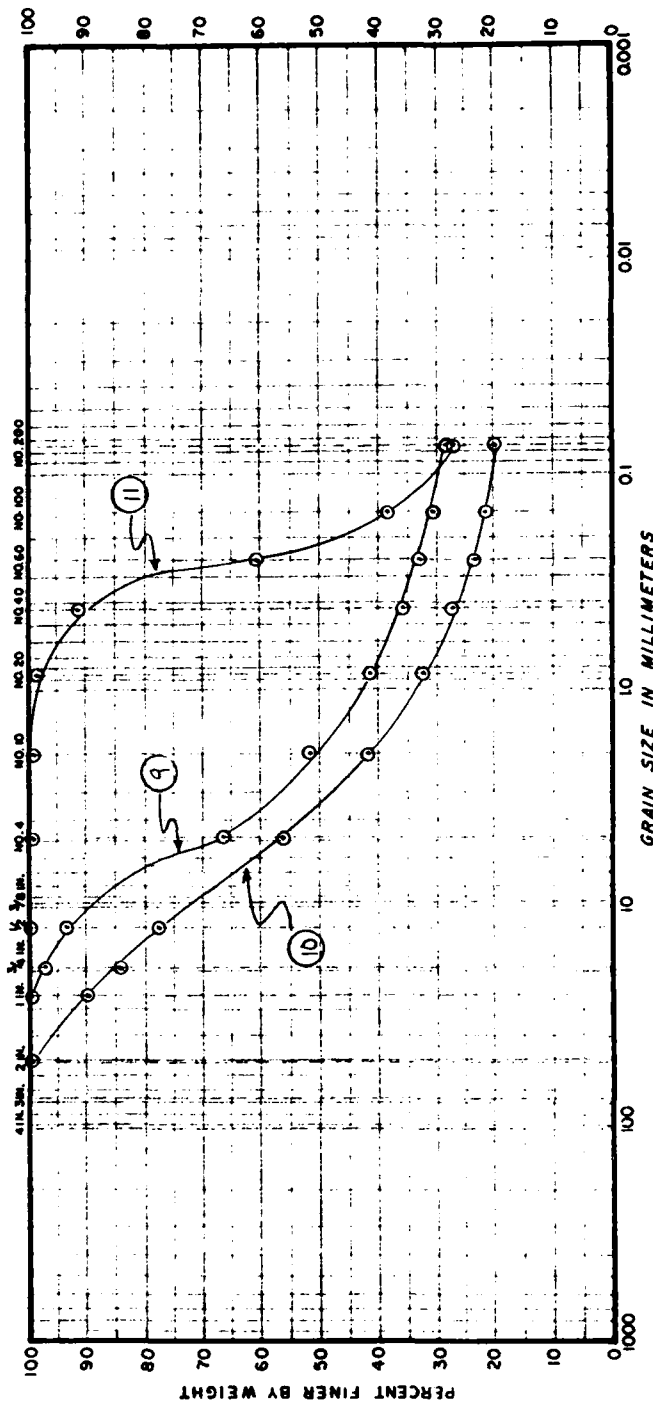
UNIFIED SOIL CLASSIFICATION SYSTEM, CORPS OF ENGINEERS, U.S. ARMY

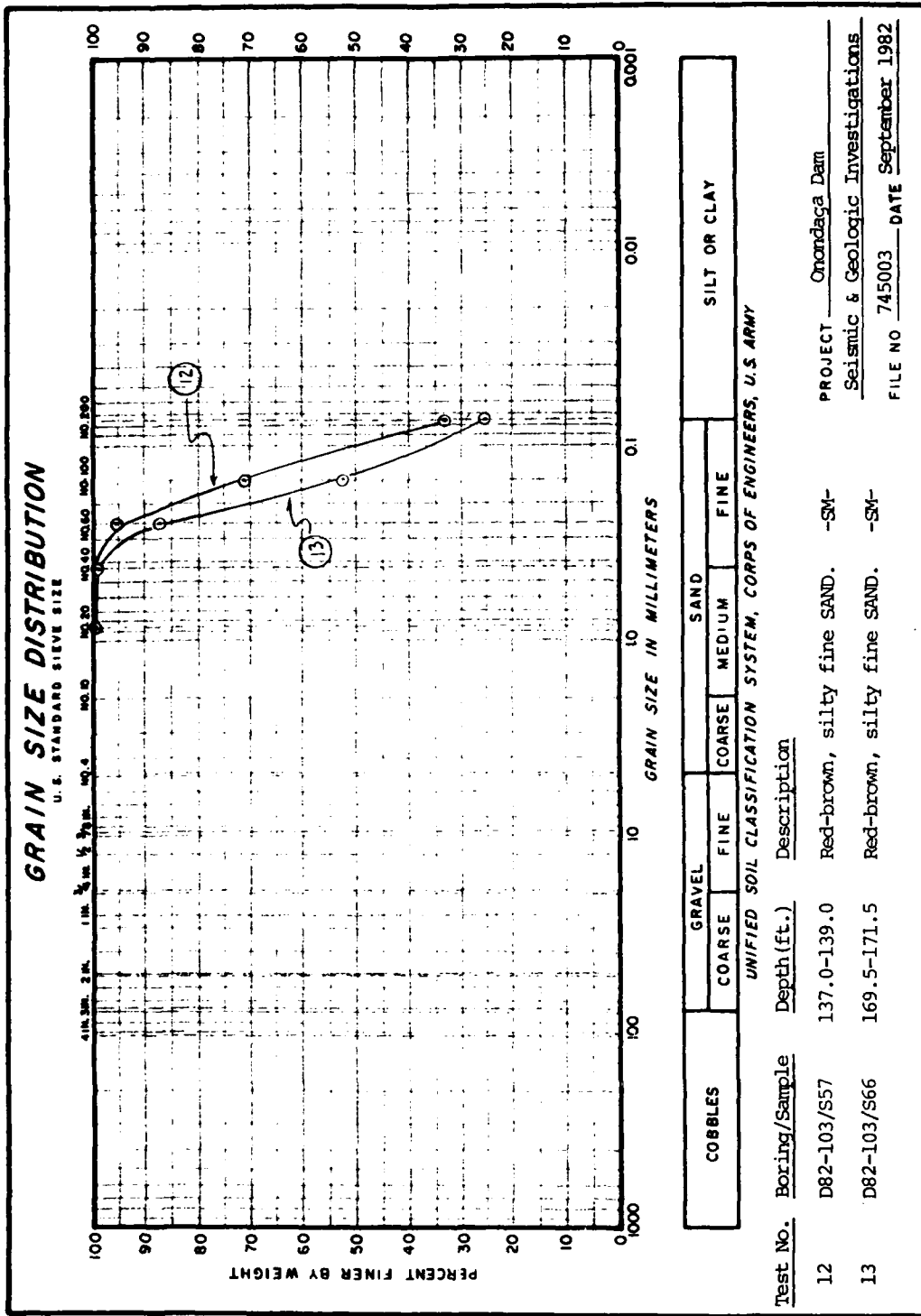
Test No.	Boring/Sample	Depth (ft.)	Description
7	D82-102/S68	106.0-108.0	Gray, coarse to fine sand, little gravel, little silt. -SM-
8	D82-102/S84	134.0-136.0	Red-brown, silty fine sand. -SM-

PROJECT Onondaga Dam
Seismic & Geologic Investigations
 FILE NO 745003 DATE August 1982

GRAIN SIZE DISTRIBUTION

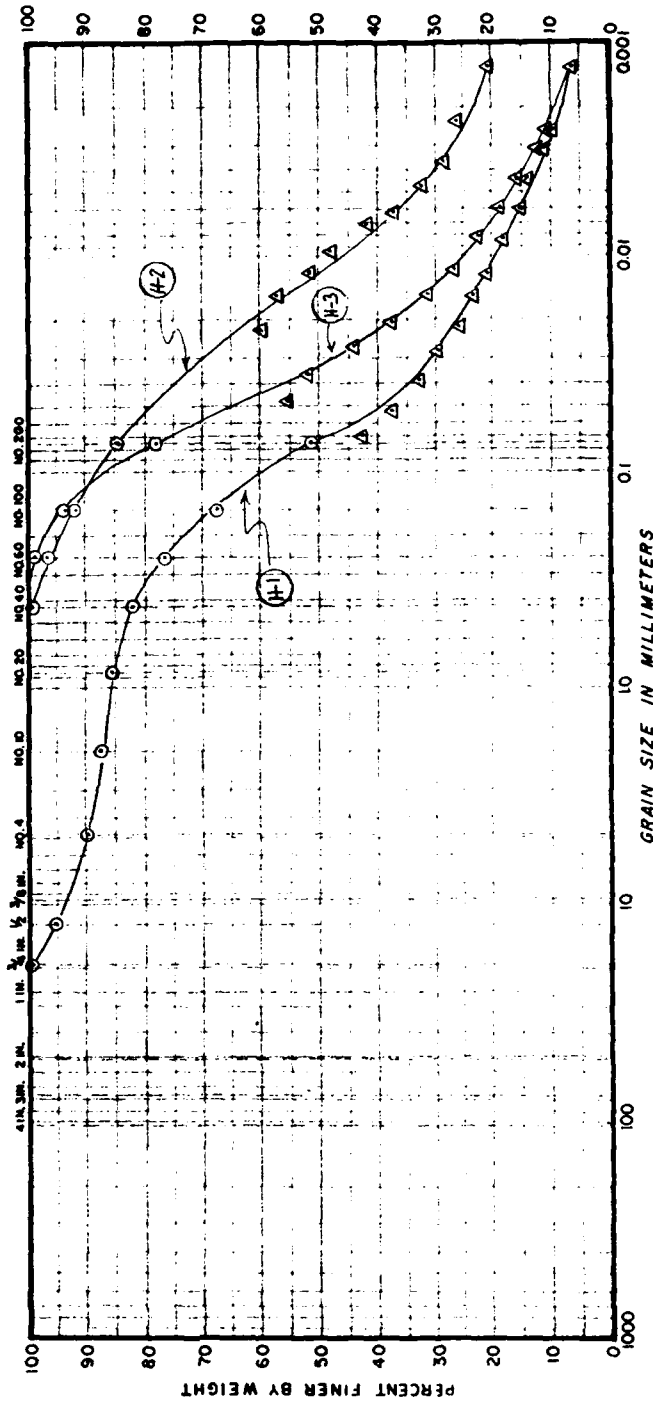
U.S. STANDARD SIEVE SIZE





GRAIN SIZE DISTRIBUTION

U. S. STANDARD SIEVE SIZE



COBBLES	GRAVEL		SAND			SILT OR CLAY
	COARSE	FINE	COARSE	MEDIUM	FINE	

UNIFIED SOIL CLASSIFICATION SYSTEM, CORPS OF ENGINEERS, U.S. ARMY

Test No.	Boring/Sample	Depth (ft.)	Description
H-1	D82-101/S17	60.0-62.0	Brown, fine sandy SILT and CLAY, trace fine gravel, trace coarse to medium sand. -CL-
H-2	D82-101/S20	64.0-65.5	Brown, silty CLAY, little fine sand. -CL-
H-3	D82-102/S38	66.0-68.0	Brown, fine sandy SILT and CLAY. -CL-

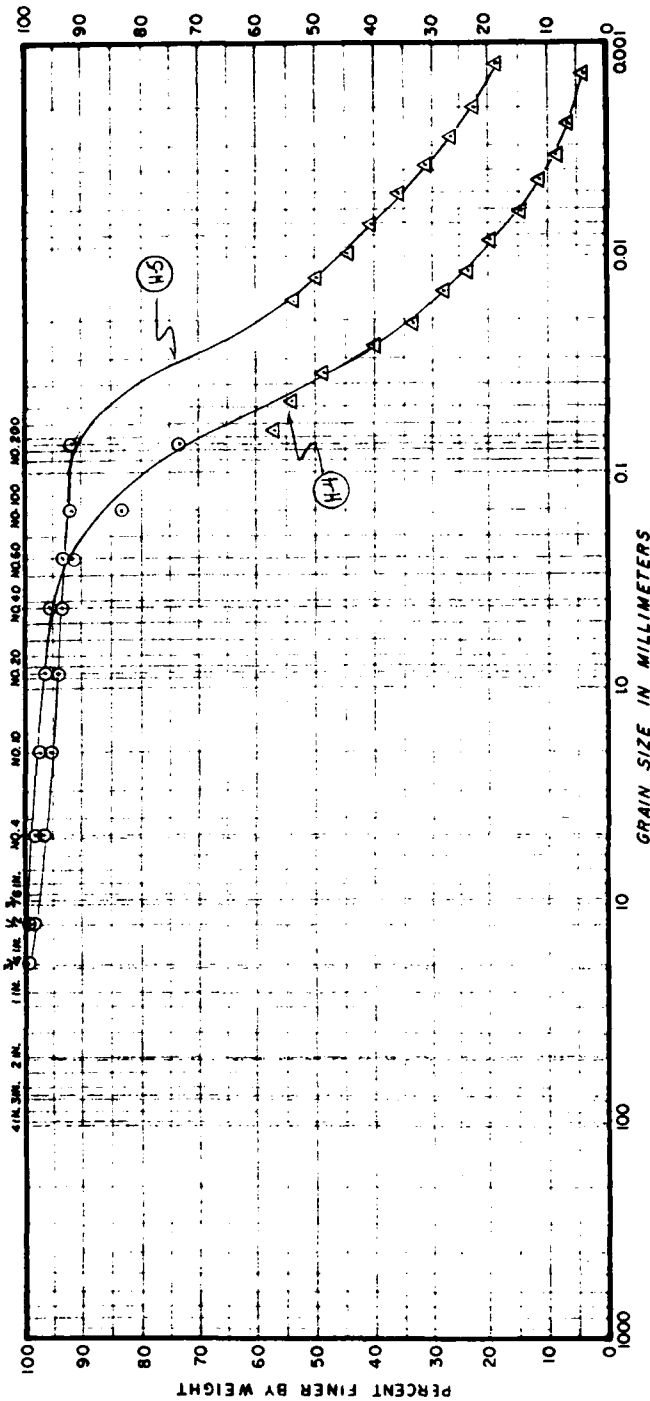
PROJECT Onondaga Dam

Seismic & Geologic Investigations

FILE NO 745003 DATE September 1982

GRAIN SIZE DISTRIBUTION

U.S. STANDARD SIEVE SIZE



CHANDLER DAM, NEW YORK

APPENDIX F

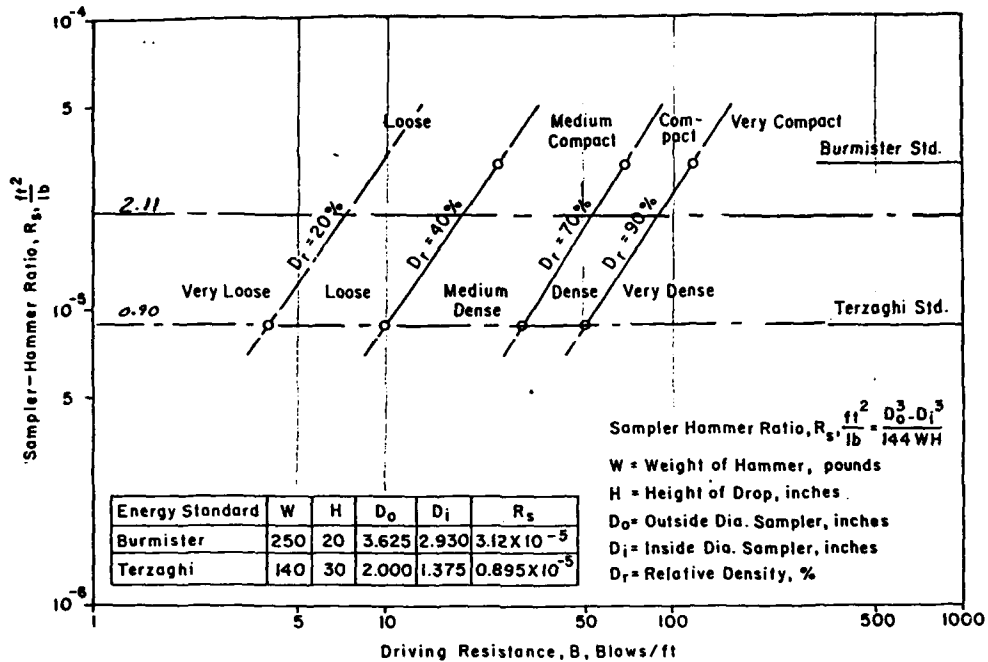
ENGINEERING RESULTS

• SAMPLER - HANDED SECTION

• LIQUEFACTION ANALYSIS

U.S. Army Corps of Engineers
New York District
New York, New York

SAMPLER-HAMMER RATIOS

Client CORPS OF ENGINEERSProject ONONDAGA DAMFile Number 745003Sheet 1 of 2Date Sept 1982Computed by DJSChecked by WJS

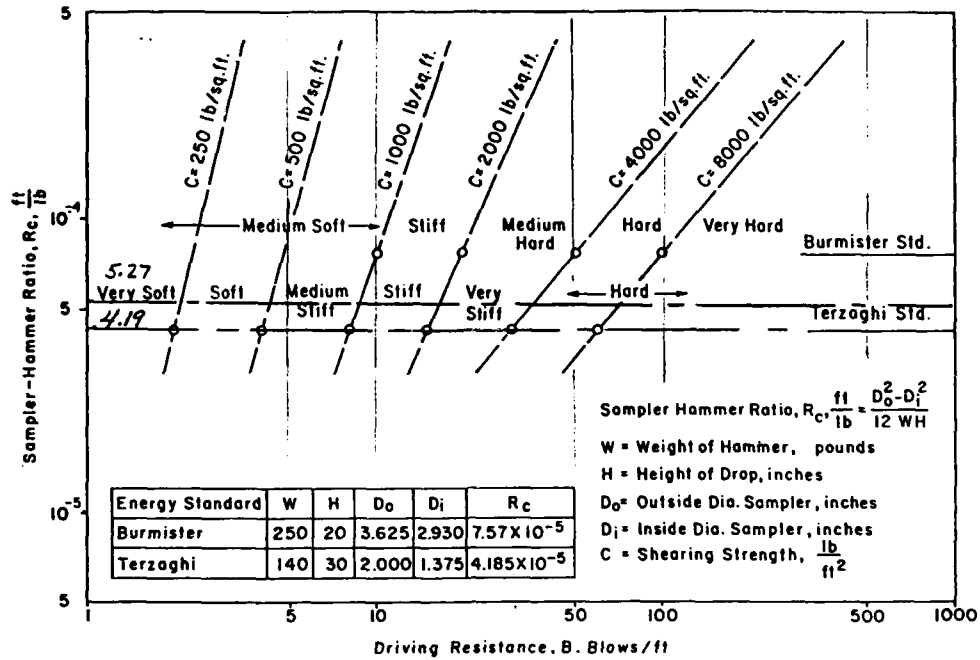
Sampler driving resistance vs compactness-cohesionless sands and silts.

SAMPLER	W (lb)	H (in)	D_o (in)	D_i (in)	R_s (ft^2/lb)
Standard	140	30	2.000	1.375	0.90×10^{-5}
Large	350	18	3.500	2.875	2.11×10^{-5}



Client **CORPS OF ENGINEERS**
Project **ONONDAGA DAM**

File Number **745003**
Sheet **2** of **2**
Date **Sept 1962**
Computed by **DTS**
Checked by **IDS**



Sampler driving resistance vs consistency-cohesive soils.

SAMPLER	W (lb.)	H (in.)	D_o (in.)	D_i (in.)	R_s (ft./lb.)
Standard	140	30	2.000	1.375	4.19×10^{-5}
Large	350	18	3.500	2.875	5.27×10^{-5}

LIQUEFACTION ANALYSIS



Client J. O. E.
 Project ONONDAGA DAM

File Number 7450-02
 Sheet SUMMARY of
 Date 9 SEP 81
 Computed by WAW
 Checked by CS

• SUMMARY OF LIQUEFACTION STUDIES FOR
ONONDAGA DAM

- SHEAR STRESS/CONFINING STRESS ($\tau/\bar{\sigma}_{v0}$) RATIOS WERE CALCULATED AS: FROM PROCEDURES BY SEED, 1976)

$$\tau/\bar{\sigma}_{v0} = 0.047 \text{ to } 0.071 \text{ BELOW TOE OF DAM}$$

$$\tau/\bar{\sigma}_{v0} = 0.040 \text{ to } 0.048 \text{ BELOW CREST OF DAM}$$

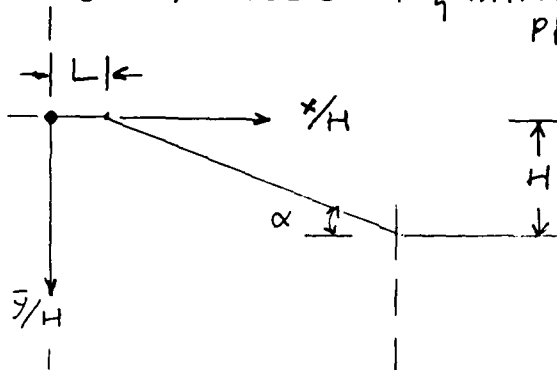
- FROM STUDIES BY SEED (1976) AND OTHERS, THE SPT-BLOW COUNT (CORRECTED TO 1 TSF OVERBURDEN PRESSURE) N_1 MUST BE LESS THAN 4.3 TO 6.5 FOR LIQUEFACTION TO BE A CONCERN FOR SOILS BELOW THE TOE OF THE DAM, AND N_1 MUST BE LESS THAN 3.6 TO 4.4 FOR SOILS BELOW THE CREST OF THE DAM TO BE CONSIDERED POTENTIALLY LIQUEFIABLE.
- ONE VALUE OF N_1 EQUAL TO 6.7 WAS FOUND IN THE FOUNDATION SOILS OF BORING DB-101 AT 5.0 FT BELOW THE DAM CREST ABOVE G.W.L. AND OTHER VALUES OF N_1 IN THE FOUNDATION SOILS EQUALED OR EXCEEDED 9.
- THEREFORE, IT IS CONCLUDED THAT THE FOUNDATION SOILS AT THE DAM SITE ARE NOT SUSCEPTIBLE TO LIQUEFACTION.



Client C. O. E.
Project ONONDAGA DAM

File Number 7450-03
Sheet 1 of 3
Date 25 AUG 82
Computed by DJS/WAW
Checked by WAW/CL

* STRESSES DUE TO EMBANKMENT
(REF. PERCOFF & BARON 1976)
pp. 602-603



• USE:
 $L = 12.5'$
 $H = 60'$
 $x/H = 0.2$

• SLOPE SLOPE = $2\frac{1}{2}:1$
 $\alpha = 22^\circ$

• ASSUME $\mu = 0.3$

a) CENTER OF EMBANKMENT $x/H = 0$

b) TOE OF EMBANKMENT $x/H = (L + H \cot \alpha) / H$

(FT) DEPTH	y/H	$\sigma_v / 18 H$ ($\sigma_v H = 60(130) = 7800$ PSF)					
		CENTER $\alpha = 15^\circ$		CENTER $\alpha = 30^\circ$		TOE $\alpha = 15^\circ$	
50	1.00	0.82	(0.76)	0.70		0.0	(0.0)
75	1.25	-		-		-	
90	1.50	0.78	(0.70)	0.61		0.07	(0.08)
105	1.75	-		-		-	
120	2.00	0.71	(0.62)	0.53		0.11	(0.12)
135	2.25	-		-		-	
150	2.50	0.66	(0.55)	0.45		0.14	(0.15)
165	2.75	-		-		-	
180	3.00	0.61	(0.50)	0.39		0.16	(0.17)

DEPTH BELOW TOP OF NATU- RAL GROUND (FT)	$\Delta \sigma_v$ BELOW DAM FROM EMBANKMENT			
	$\Delta \sigma_v$ (PSF) (C)		$\Delta \sigma_v$ (PSF) (TOE)	
	55' EMB.	55' EMB.	55' EMB.	65' EMB.
0	5435	6,420 (6190)	0	0
30	5005	5915 (5700)	575	675
60	4435	5240 (5045)	823	1,015
90	3935	4650 (4480)	1,075	1,270
120	3575	4230 (4070)	1,215	1,440
		\uparrow GWT @ 65' (NATURAL)		



Client C.O.E.
Project ONONDAGA DAM

File Number 7450-57
Sheet 2 of 3
Date 6 SEP 82
Computed by WAW
Checked by CS

* DYNAMIC SHEAR STRESSES

$$\tau = 0.05 a_{max} r_d \sigma_{vo}$$

a_{max} = peak horizontal ground acceleration at ground surface (g)

r_d = depth reduction factor

σ_{vo} = total overburden pressure

* REFERENCE ATC-3 DOCUMENT - FIG. C1-8

- RATIO OF "STIFF SOIL" PEAK TO "ROCK" PEAK :

$$\frac{2.5}{2.2} = 1.14$$

- USE 1.2 AS AMPLIFICATION FACTOR.

- NATURAL GROUND SURFACE ACCELERATION (PEAK HORIZONTAL) = $(1.2)(0.05) = 0.06 g$

$$a_{max} = 0.06 g$$

DEPTH (FT)	σ_{vo} (PSF)	BELOW TOE OF DAM (NATURAL GROUND)		BELOW CREST OF DAM	
		r_d	τ (PSF)	r_d	τ (PSF)
1	150	1.0	0		
5	510	1.0	26		
10	1000	0.90	50		
20	2000	0.75	77		
30	3000	0.6	100		
40	4000	0.52	122		
50	5000	0.45	146	1.0	260
75	7500	0.3	216	0.9	351
100	10000	0.2	296	0.8	416
125	12500	0.1	380	0.7	520
150	15000	0.0	464		



Client C. O. E
Project ONONDAGA DAM

File Number 7450-03
Sheet 3
Date SEP 8 '61
Computed by WAW
Checked by CS

* $\frac{v}{\sigma_v}$ RATIOS									
BORING DEPT TOP OF DAM (FT)	DEPT BELOW NATURAL GROUND (FT)	σ_v (PSF)		σ_v (PSF)		$\frac{v}{\sigma_v}$		* N	* C _N
		REST	TOE	REST	TOE	REST	TOE		
102-									
101	56	150	150	7535	150	223	7	0.040	0.017
4WT@	5	6130	895	6065	835	245	35	0.040	0.042
52.465	71	7265	2385	6600	1705	282	89	0.043	0.052
(60')	77	7980	3180	6925	2225	301	119	0.044	0.057
	97	10,235	6150	7945	3855	361	206	0.045	0.053
AT NATURAL GROUND, $\sigma_v = 61420$ PSF, $\bar{\sigma}_v = 6190$ PSF									
102	47	6645	305	6295	180	262	13	0.042	0.071
4WT@	4	6870	610	6395	360	270	24	0.042	0.068
52.465	105	10,890	5990	8200	3480	374	201	0.046	0.058
(60')	121	12,610	8250	8940	4740	408	261	0.046	0.055
	142	14,915	11,170	9960	6350	477	357	0.048	0.056
102	5	6265	765	6450	4350	270	30	0.042	0.067
4WT@	38	10,375	5765	8110	3320	370	195	0.046	0.058
52.465	65	13,590	9510	9370	5735	435	305	0.046	0.056
(60')									

* VALUES ADDED BY BUFFALO DIST BY GL 616183
() REPRESENTS ACTUAL UNCORRECTED PENETRATION
RESISTANCES FOR 3/8" DIA. SPOON.

END

DATE
FILMED

8-86